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MR&I OFFICE REMODE

RURAL WATER SYSTEM



DRAWING NO

G-001

Bartlett & West

DRAWING NUMBER

G-001

G-002

G-101

G-102

G-103

S101

S201

S202

S301

S302

S401

S501

A-011

A-101

A-111

A-121

A-131

A-201

A-301

A-601

A-701

MEP000

P-101

P-102

P-111

P-112

P-501

M-101

M-102

M-111

M-112

M-501

E-101

E-102

E-103

E-104

E-111

E-112

E-113

E-114

E-501

E-601

COVER

CODE PLAN

CODE STUDY

MOUNTING

ROOF FRAMING PLAN

FOUNDATION DETAILS

FRAMING DETAILS

DEMOLITION PLAN

REFLECTED CEILING PLAN

SITE PLAN

FLOOR PLAN

ROOF PLAN

ELEVATIONS

SECTIONS

SCHEDULES

ROOM ELEVATIONS

MEP SYMBOLS & ABBREVIATIONS

PLUMBING DEMOLITION PLAN

PLUMBING IMPROVEMENT PLAN

MECHANICAL DEMOLITION PLAN

MECHANICAL IMPROVEMENT PLAN

LIGHTING DEMOLITION PLAN

FIRE ALARM DEMOLITION PLAN

LIGHTING IMPROVEMENT PLAN

FIRE ALARM IMPROVEMENT PLAN

SPECIAL SYSTEMS IMPROVEMENT PLAN

POWER IMPROVEMENT PLAN

ELECTRICAL DETAILS

ELECTRICAL SCHEDULES

SPECIAL SYSTEMS DEMOLITION PLAN

POWER DEMOLITION PLAN

MECHANICAL DETAILS & SCHEDULES

PLUMBING UNDERFLOOR DEMOLITION PLAN

PLUMBING UNDERFLOOR IMPROVEMENT PLAN

PLUMBING DETAILS, SCHEDULES & RISERS

MECHANICAL UNDERFLOOR DEMOLITION PLAN

MECHANICAL UNDERFLOOR IMPROVEMENT PLAN

VESTIBULE ROOF FRAMING PLAN

GENERAL STRUCTURAL NOTES

SITE LOCATION & DRAWING INDEX

TYPICAL WALL ASSEMBLIES, DRAWING STANDARDS, AND

FOUNDATION PLAN & SPECIAL INSPECTION TABLES

TYP. FOUNDATION DETAILS & PIER DETAILS

artlett&West

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OCATION & DRAWING INDE

MR&I OFFICE REMOD

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PRAWN BY:
PESIGN PROJ:
PESIGN P

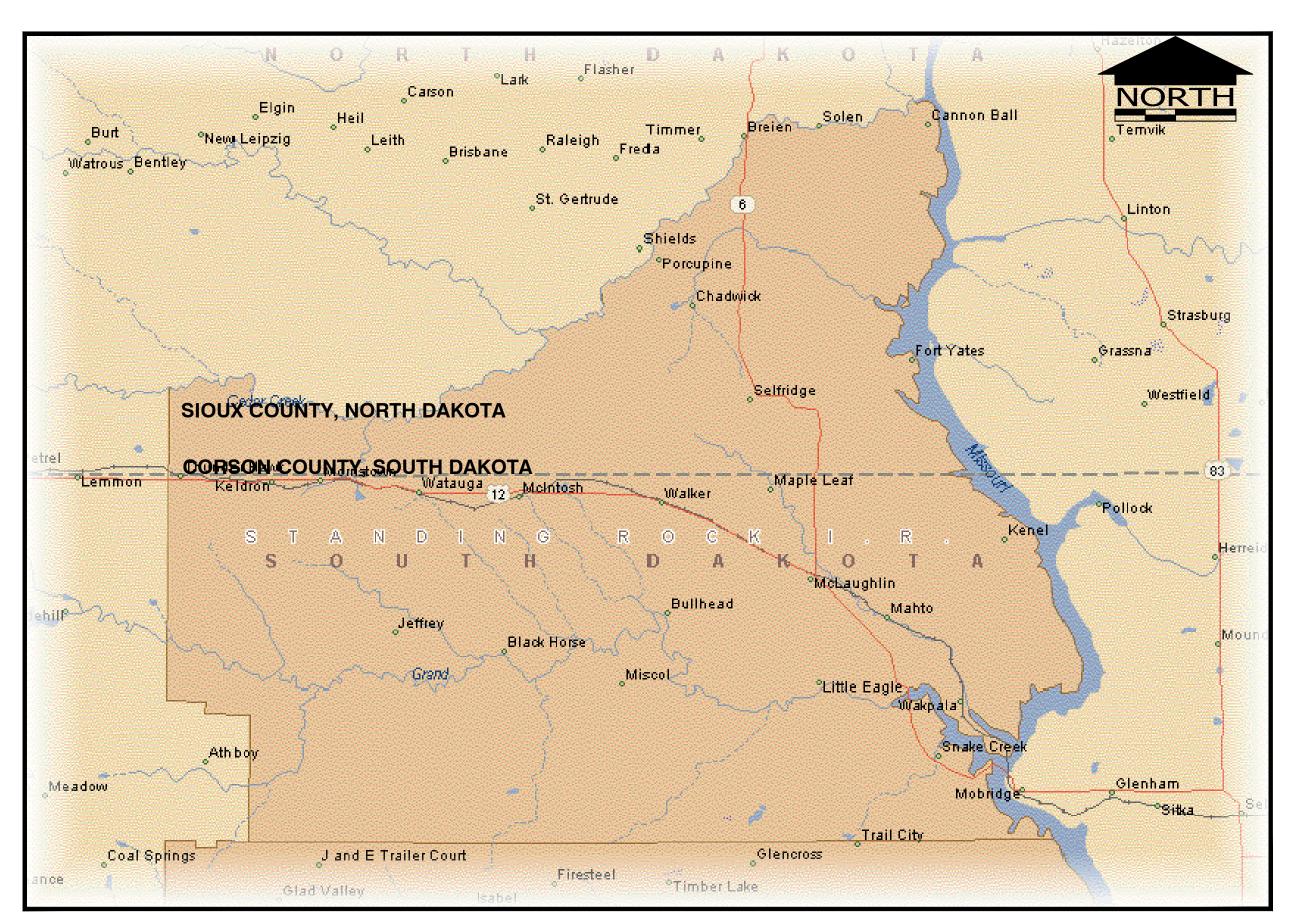
G-002

STANDING ROCK MR&I PROJECT SITE

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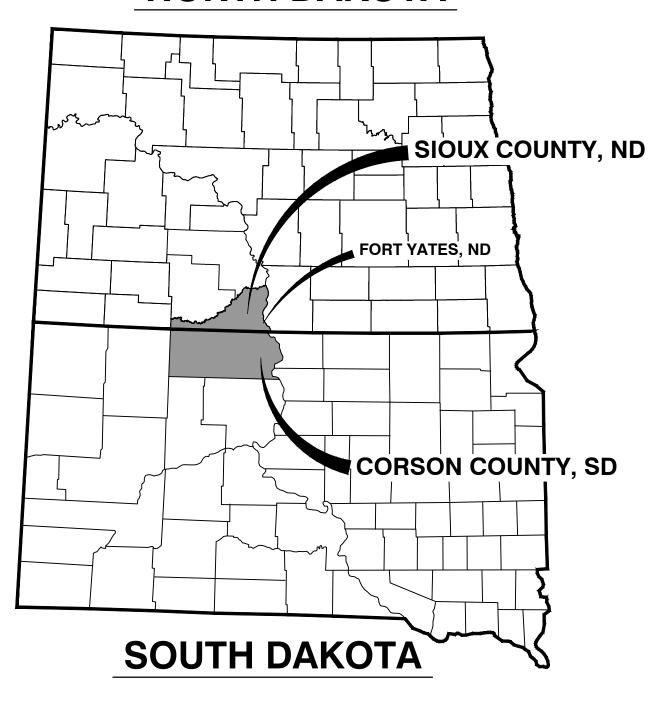
PROJECT LOCATION

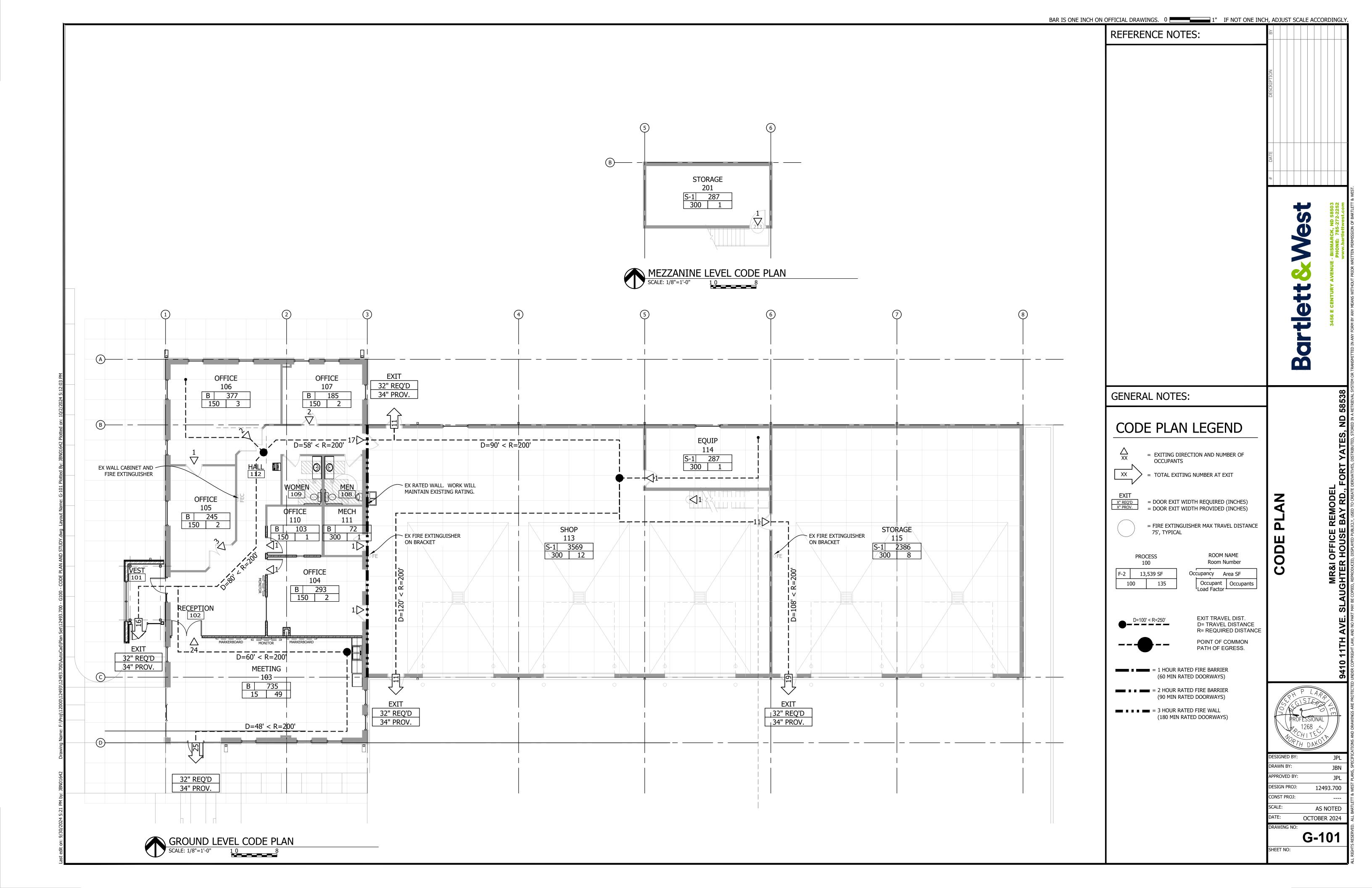
STANDING ROCK RESERVATION



STANDING ROCK RESERVATION

NORTH DAKOTA



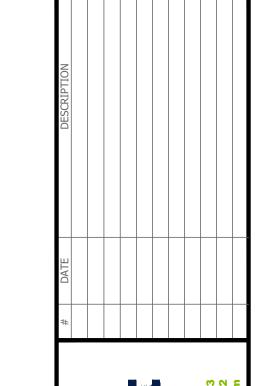


Travel Distance (1017)

Maximum exit access travel distance (feet) (w/o sprinkler system) B
Maximum exit access travel distance (feet) (w/o sprinkler system) S-1

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		was piid	n to the an	eration.							
pter 6	- Classifi	cation of	Work								
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		602.1 Scc		ation Leve							
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		Section 6	103 - Alfor	ation Leve	12						
		603.1 Sco		ation Leve	. 1 4						
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pter 7	- Alterati	ons Level	1								
		Section 7701.2 Cor	701 - Gene	ral							
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	Section 803 - Fire Protection 803.1 Scope
	The requirements of this section shall be limited to the work areas in which Level 2 alterations are
	being performed, and where specified they shall apply throughout the floor on which the work areas
	are located or otherwise beyond the work area.
	803.4 Fire Alarm and detection
	An approved automatic fire detection system shall be installed in accordance with the provisions of
	this code and NFPA72. Devices, combinations of devices, appliances, and equipment shall be
	approved. The automatic fire detectors shall be smoke detectors, except that an approved alternative type of detector shall be installed in spaces such as boiler rooms, where products of combustion are
	present during normal operations in sufficient quantity to actuate a smoke detector.
	903 4.1 Occupancy Poquiroments
	803.4.1 Occupancy Requirements Where the building is not equipped with a fire alarm system, alarm-notification appliances within the
	work area shall be provided and automatically activated.
	Section 804 - Means of Egress 804.1 Scope
	The requirements of this section shall be limited to work areas in which level 2 alterations are being
	performed, and where specified they shall apply throughout the floor on which the work areas are
	located or otherwise beyond the work area.
	804.4 Number of exits.
	The number of exits shall be in accordance with Section 804.4.1 through 804.4.3
	804.4.1 Minimum number. Every story utilized for human occupancy on which there is a work area that includes exits shall be
	provided with the minimum number of exits based on the occupancy and occupant load in accordance
	with the IBC.
	804.5 Egress Doorways
	804.5.1.1 In any work area, all rooms and spaces having an occupancy load greater than 50 or in
	which the travel distance to an exit exceeds 75 feet shall have not fewer than two egress doorways.
	204 F 2 Da as Outlines
	804.5.2 Door Swing In the work area and in the egress path from any work area to the exit discharge, all egress doors
	serving an occupant load greater than 50 shall swing in the direction of travel.
	804.9 Exit Signs Means of egress in all work areas shall be provided with exit signs in accordance with the
	requirements of the IBC
	Section 806 - Electrical
	806.1 New Installations Newly installed electrical equipment and wiring relating to work done in any work area shall comply
	with all applicable requirements of NFPA 70.
	Section 807 - Mechanical 807.1 Reconfigured or converted spaces.
	007.1 Neconligured of converted spaces.
	Reconfigured spaces intended for occupancy and spaces converted to habitable or occupiable space
	in any work area shall be provided with natural or mechanical ventilation in accordance with the IMC.
	807.2 Altered Existing Systems.
	In mechanically ventilated spaces, existing mechanical ventilation systems that are altered,
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	reconfigured, or extended shall provide not less than 5 cubic feet per minute (cfm) per person of
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BAR IS ONE INCH ON OFFICIAL DRAWINGS. 0 _______ 1" IF NOT ONE INCH, ADJUST SCALE ACCORDINGLY.

GE CENTURY AVENUE - BISMARCK, ND 58

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SA CIN SET AY TO

AR&I OFFICE REMODEL

ARA HILL DATE SI O



DESIGNED BY: JPL
DRAWN BY: JBN
APPROVED BY: JPL
DESIGN PROJ: 12493.700
CONST PROJ: ----

AS NOTED

DATE: OCTOBER 2024

G-102

EET NO:

SEE STRUCTURAL FOR ANCHORAGE REQ'RMTS.

COMPRESSIBLE DECK FLUTE INSERTS-SEE SECT 079200.

- DEFLECTION TRACK - SEE SECT

092116 FOR TYPE, LOCATION,

TERMINATE WALL STUDS 1"

AND FASTENER.

NON-RATED PARTITION HEAD

RATED PARTITION HEAD

PREFORMED FIRESTOP —

SEAL-SEE SECT 092116.

SCALE: 3"=1'-0"

MIN FROM T/TRACK.

SEE WALL TYPE FOR

ANCHORAGE REQ'RMTS.

ADDITIONAL LAPS AND

OF UNDERSIDE OF DECK.

FIRESTOP TO FOLLOW PROFILE

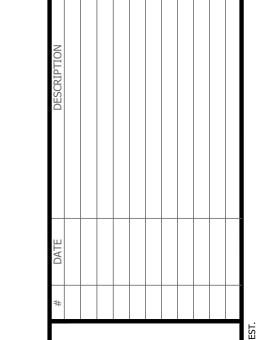
- DEFLECTION TRACK - SEE SECT

092116 FOR TYPE, LOCATION,

- TERMINATE WALL STUDS 1" MIN FROM T/TRACK.

AND FASTENER.





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TYPICAL ASSEMBLIESTANDARDS, AND MOUN

ROBE OR COAT

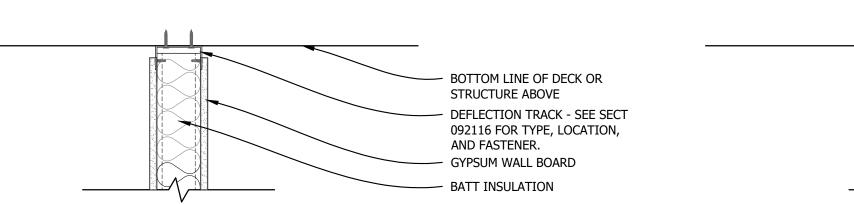
STAIR HANDRAIL

HOOK

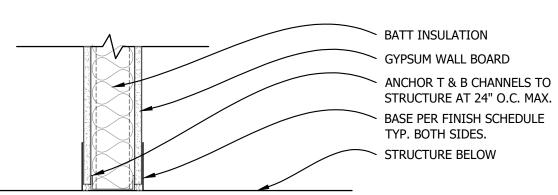
PT	H DAKOTA
DESIGNED BY:	JPL
DRAWN BY:	JBN
APPROVED BY:	JPL
DESIGN PROJ:	12493.700
CONST PROJ:	
SCALE:	AS NOTED
DATF:	OCTOBED 2024

OCTOBER 2024

G-103

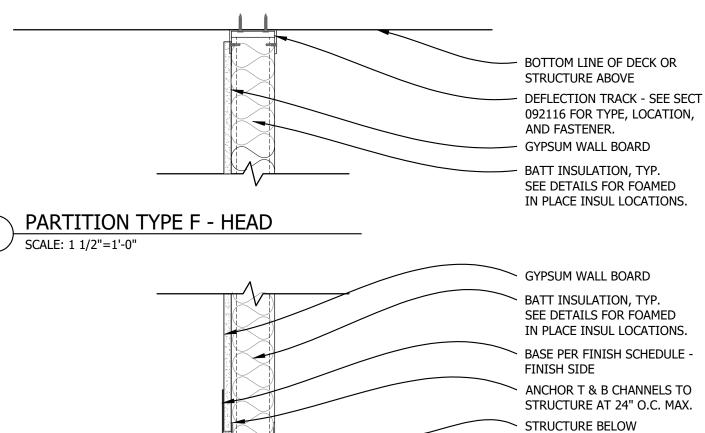


PARTITION TYPE A - HEAD SCALE: 1 1/2"=1'-0"



PARTITION TYPE A - SILL SCALE: 1 1/2"=1'-0"

PARTITION TYPE A							
WALL TAG	STUD SIZE & SPACING	WIDTH	G.B. THK (EACH SIDE)	INSUL THICK	RATING	UL NO.	COMMENTS
A4	3 5/8" M.S. @ 16" O.C.	4 7/8"	5/8"	FILL CAVITY	1 HR	U404	SEE CODE PLAN FOR FIRE WALL LOCATIONS
A6	6" M.S. @ 16" O.C.	7 1/4"	5/8"	FILL CAVITY	1 HR	U419	SEE CODE PLAN FOR FIRE WALL LOCATIONS



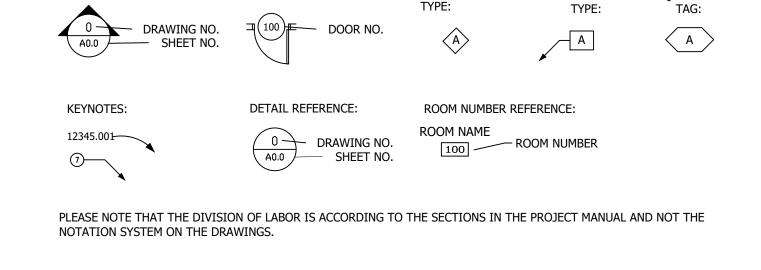
PARTITION TYPE F - SILL SCALE: 1 1/2"=1'-0"

	PARTITION TYPE F						
WALL TAG	STUD SIZE & SPACING	WIDTH	G.B. THK (ONE SIDE)	INSUL THICK	RATING	UL NO.	COMMENTS
F2	1 5/8" M.S. @ 16" O.C.	2 1/4"	5/8"	FILL CAVITY	1 HR	U445	SEAL PENT. W/ ACOUSTICAL SEALANT
F3	2 1/2" M.S. @ 16" O.C.	3 1/8"	5/8"	FILL CAVITY	1 HR	U448	SEAL PENT. W/ ACOUSTICAL SEALANT
F4	3 5/8" M.S. @ 16" O.C.	4 1/4"	5/8"	FILL CAVITY	1 HR	U445	SEAL PENT. W/ ACOUSTICAL SEALANT
F6	6" M.S. @ 16" O.C.	6 5/8"	5/8"	FILL CAVITY	1 HR	U445	SEAL PENT. W/ ACOUSTICAL SEALANT



(DIMENSIONS SHOWN ELSEWHERE FOR SPECIFIC MECH/ELECT ITEMS WILL SUPERSEDE THOSE REFERRED TO HERE)

HANDICAP URINAL



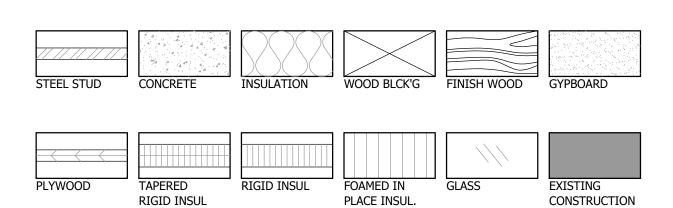
WINDOW/FRAME

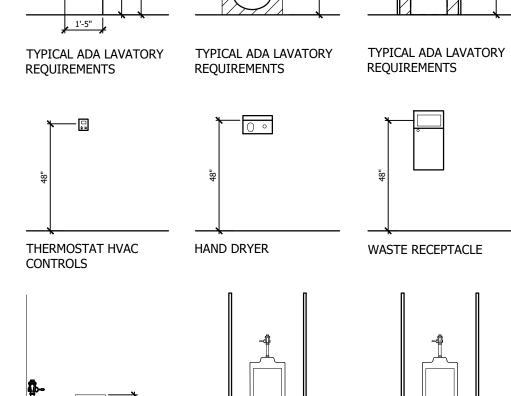
EQUIPMENT

MATERIAL INDEX

DRAWING SYMBOLS

ELEVATIONS AND SECTIONS: OPENING TYPE:

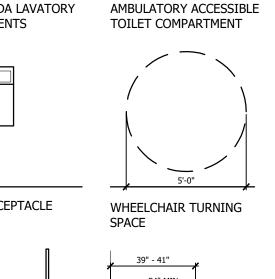




TYPICAL URINAL

FEMININE NAPKIN

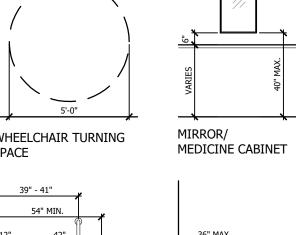
DISPOSAL

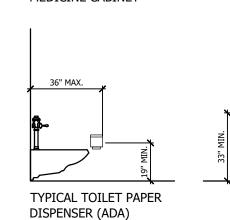


GRAB BAR

/ TOWEL BAR

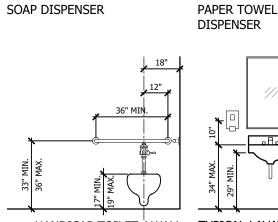
5'-0"



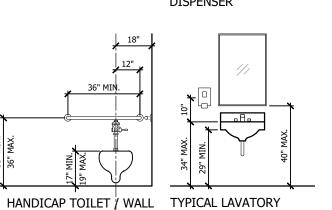


WHEELCHAIR ACCESSIBLE

TOILET COMPARTMENT



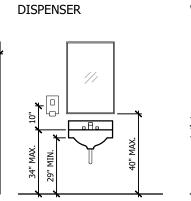
DRINKING FOUNTAIN

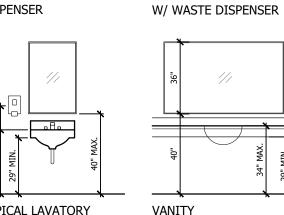


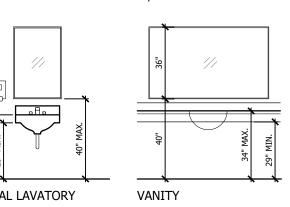
NOTE: CONTROLS SHALL BE FRONT OR SIDE MOUNTED NEAR FRONT EDGE. PROVIDE 30"X48" CLR. FLR. SPACE FOR FRONT APPOACH.

ELECTRIC WATER

COOLER (ADA)







FIRE ALARM PULL

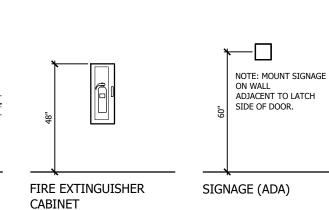
PAPER TOWEL CABINET

STATION

E DISPENSE	ER .	PARTITIC	Ν
///		*	
34" MAX.	29" MIN.	48"	
٠,	. 	FIRE EXT	TNG

MIRROR

TOILET



HOOKS/SHELF

MOP OR BROOM

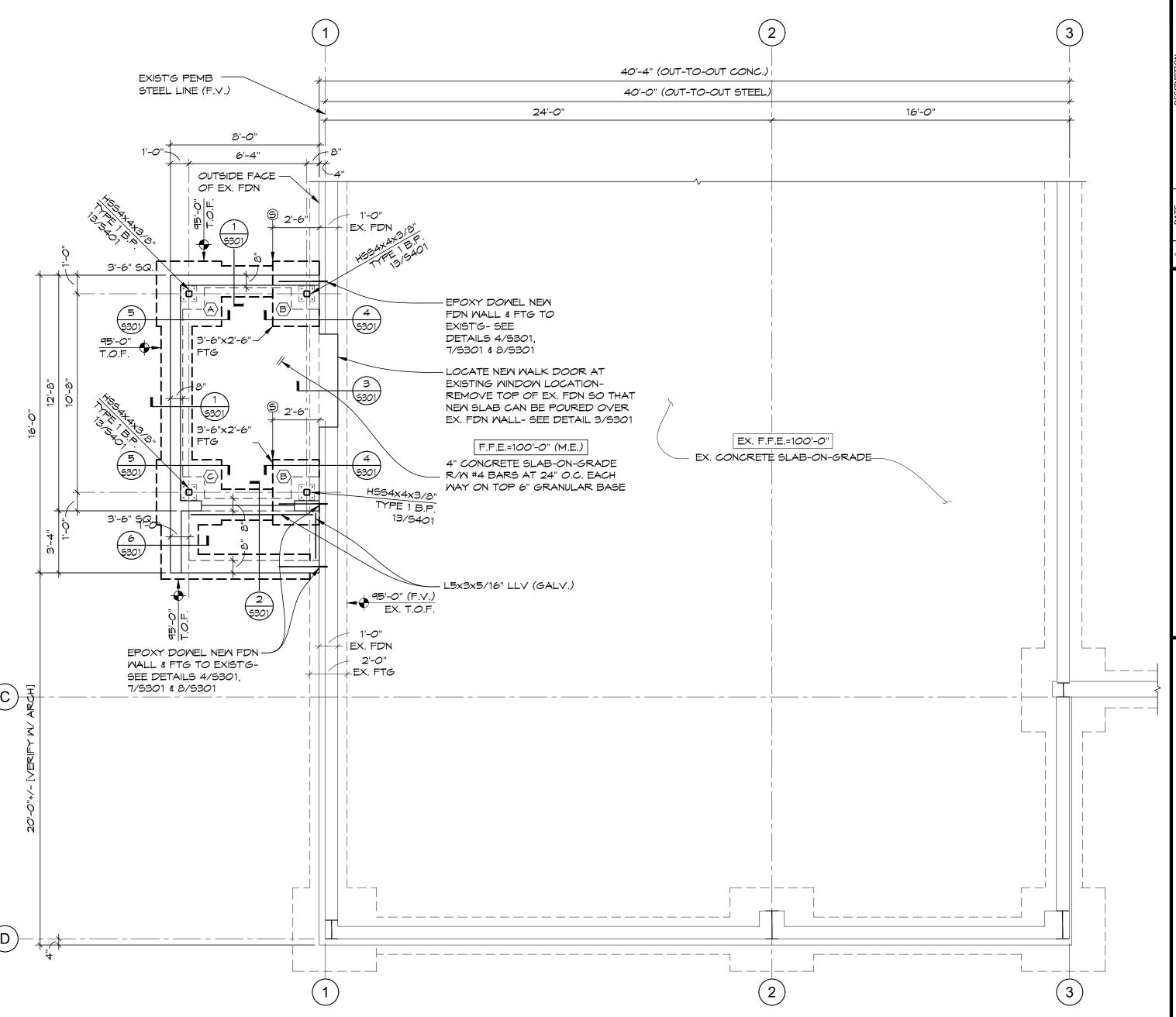
HOLDER

	(IBC Table 1	1705.3)	
	FREQUI	ENCY	
	Continuous	Periodic	Comments:
1) Inspect Reinforcement			
Inspect reinforcement, including prestressing tendons, and verify placement.		•	Verify reinforcing and placement as shown on drawing and per ACI 318 Ch. 20, 25.2, 25.3, 26.6.1-26.6.3 & IBC 1908.4
2) Reinforcing Bar Welding:		1	
Verify weldability of reinforcing bars other than ASTM A706:		•	Verify bar welding per AWS D1.4 & ACI 318: 26.6.4
Inspect single-pass fillet welds, maximum 5/16th; and		•	
Inspect all other welds.	•		
3) Inspect anchors cast in concrete.		+	Verify anchors are cast in concrete per structural drawings and ACI 318: 17.8.2
4) Inspect anchors post-installed in	hardened concret	te members b:	
Adhesive anchors installed in horizontally or upwardly inclined orientations to resist sustained tension loads.	•		Verify anchors are post installed in concrete per structural drawings, ACI 318: 17.8.2.4 (adhesive
Mechanical anchors and adhesive anchors not defined in 4.a.		•	anchors) & 17.8.2 (mechanical anchors)
5) Verify use of required design mix.		•	Verify mix designs meet strength and exposure requirements listed on approved plans; verify compliance with ACI 318: Ch. 19, 26.4.3, 26.4.4, & IBC 1904.1, 1904.2, 1908.2, 1908.3
6) Prior to concrete placement, fabricate specimens for strength tests, perform slump and air content tests, and determine the temperature of the concrete.	•		Verify, perform, & examine tests per ASTM C172, AST C31, ACI 318: 26.4 & 26.12, and IBC 1908.10
7) Inspect concrete placement & application techniques.	•		Inspect concrete & shotcrete placement for proper application per ACI 318: 26.5 & IBC 1908.6, 1908.7, & 1908.8
8) Verify curing temperature and techniques.		•	Verify concrete curing temperature and technique is being maintained per approved plans, ACI 318: 26.5.3-26.5.5, & IBC 1908.9
9) Inspect prestressed concrete for	:	!	
Application of prestressing forces: and	•		Verify prestressed concrete per ACI 318: 26.10
Grouting of bonded prestressing tendons.	•		
10) Inspect erection of precast concrete.		•	Inspect erection per ACI 318: Ch 26.8
11) Verify in-situ concrete strength, prior to stressing of tendons in post-tensioned concrete and prior to removal of shores and forms from beams and structural slabs.		•	Verify strength per ACI 318: 26.11.2
12) Inspect formwork for shape, location and dimensions of the concrete member being formed.		•	Inspect formwork for shape, locations, and dimension of concrete member being formed per approved drawings and ACI 318: 26.11.1.2(b)

(a) Where applicable, see also Section 1705.12, Special inspections for seismic resistance.

REQUIRED SPI	ECIAL INSPECTION	ONS AND TES	STS OF SOILS	
(IBC 20	15 Section 1705	6.6 & Table 1	705.6)	
	FREQUI	NCY		
	Continuous	Periodic	Comments:	
1) Minimum Testing:		•		
Verify Materials below shallow foundations are adequate to achieve the design bearing capacity.		•		
2) Prior to Construction:				
Verify excavations are extended to proper depth and have reached proper material.		+		
3) As Construction Begins:		•	•	
Perform classification and testing of compacted fill material.		•		
4) During Construction:			•	
Verify use of proper materials, densities and lift thicknesses during placement and compaction of compacted fill.	*			
Prior to placement of compacted fill, inspect subgrade and verify that site has been prepared properly.		•		

	FREQUE	ENCY	
	Continuous	Periodic	Comments:
OTHER THAN STRUCTURAL STEEL (IBC 1705.11.3)		
Steel Roof & Floor Deck: (SDI -QA/	QC)		
Material verification of steel deck		•	Identification markings per applicable ASTM standard.
Roof and Floor deck welds		•	Verify that welds conform to AWS D1.3.
nstallation of mechanical fasteners		•	Verify that fasteners conform to SDI C, SDI NC, SDI RD & manufacturer's instruction as well as construction documents
STRUCTURAL STEEL CONSTRUCTIO	N (IBC 1705.2, 170	 5.11, 1705.12)	
Prior to Welding (Table N5.4-1, AIS	SC 360-16):		
Verify welding procedures	•		
Material identification		•	Verify type and grade of material.
Welder identification		•	Verify there is a system in place to identify the welder
		•	who has welded a joint or member.
Fit-up groove welds		•	Verify joint preparation, dimensions, cleanliness, tacking and backing.
Access holes		•	Verify configuration and finish.
Fit-up fillet welds		•	Verify alignment, gaps at root, cleanliness of steel surfaces, tack weld quality and location.
During Welding (Table N5.4-02, Als	C 360-16):	<u>l</u>	
Use of qualified inspectors		•	Verify that welders are appropriately qualified.
Control and handling of welding consumables		•	Verify that weiters are appropriately qualified. Verify packaging and exposure control.
Cracked tack welds		•	Verify welding is not over a cracked tack weld.
Environmental conditions			Verify wind speed is within limits as well as precipitation
		•	and temperature.
WPS followed		•	Verify items such as welding equipment settings, travel speed, welding materials, shielding gas type/flow rate, preheat applied, interpass temperature maintained, and proper position.
Welding techniques		•	Verify interpass and final cleaning, each pass is within profile limitations, and quality of each pass.
Steel headed studs anchors atop beams			Verify placement and installation of steel headed stud
steer readed stads arteriors atop bearins	•		anchors. See general structural notes for additional commentary of the testing of headed stud anchors
After Welding (Table N5.4-3, AISC	360-16):		
Welds cleaned		•	Verify that welds have been properly cleaned.
Size, length and location of welds	•		
Welds meet visual acceptance criteria	•		
Arc strikes	•		
k-area	•		
Backing & welding tabs removed	•		
Repair activities	•		
Document acceptance/rejection of weld	•		
No prohibited welds have been added without the approval		•	
of the EOR	0.200.46\	•	
Nondestructive Testing (N5.5, AIS	C 360-16):		Turi de la constanta de la con
CIP welds (Risk Cat. II)			Ultrasonic testing shall be performed on 10% of CIP groove welds in butt, T-and corner joints subject to transversely applied tension loading in materials 5/16-inch thick or greater. Testing rate must be
		•	increased if > 5% of welds have unacceptable defects.
CIP welds (Risk Cat. III or IV)		•	increased if > 5% of welds have unacceptable defects. Ultrasonic testing shall be performed on 100% of CJP groove welds subjected to transversely applied tension loading in butt, T-and corner joints, in materials 5/16-inch thick or greater. Testing rate must be
	•	•	increased if > 5% of welds have unacceptable defects. Ultrasonic testing shall be performed on 100% of CJP groove welds subjected to transversely applied tension loading in butt, T-and corner joints, in materials
Access holes (flange > 2")		•	increased if > 5% of welds have unacceptable defects. Ultrasonic testing shall be performed on 100% of CJP groove welds subjected to transversely applied tension loading in butt, T-and corner joints, in materials 5/16-inch thick or greater. Testing rate must be
Access holes (flange > 2") Welded Joints subject to fatigue	+	Nes 18-1 and 11	increased if > 5% of welds have unacceptable defects. Ultrasonic testing shall be performed on 100% of CJP groove welds subjected to transversely applied tension loading in butt, T-and corner joints, in materials 5/16-inch thick or greater. Testing rate must be increased if > 5% of welds have unacceptable defects.
Access holes (flange > 2") Welded Joints subject to fatigue Other Steel Inspections (Table N5.	+	ples J8-1 and J	increased if > 5% of welds have unacceptable defects. Ultrasonic testing shall be performed on 100% of CIP groove welds subjected to transversely applied tension loading in butt, T-and corner joints, in materials 5/16-inch thick or greater. Testing rate must be increased if > 5% of welds have unacceptable defects. LO-1, AISC 341-10):
Access holes (flange > 2") Welded Joints subject to fatigue Other Steel Inspections (Table N5.) Structural steel details	+	ples J8-1 and J1	increased if > 5% of welds have unacceptable defects. Ultrasonic testing shall be performed on 100% of CIP groove welds subjected to transversely applied tension loading in butt, T-and corner joints, in materials 5/16-inch thick or greater. Testing rate must be increased if > 5% of welds have unacceptable defects. LO-1, AISC 341-10): All fabricated steel and their connections shall be inspected to verify compliance with the details shown in the approved plans.
Access holes (flange > 2") Welded Joints subject to fatigue Other Steel Inspections (Table N5.) Structural steel details	+	oles J8-1 and J1	increased if > 5% of welds have unacceptable defects. Ultrasonic testing shall be performed on 100% of CJP groove welds subjected to transversely applied tension loading in butt, T-and corner joints, in materials 5/16-inch thick or greater. Testing rate must be increased if > 5% of welds have unacceptable defects. 10-1, AISC 341-10): All fabricated steel and their connections shall be inspected to verify compliance with the details shown in
Access holes (flange > 2") Welded Joints subject to fatigue Other Steel Inspections (Table N5.) Structural steel details Anchor rods/embeds supporting structural steel	+	oles J8-1 and J1	Ultrasonic testing shall be performed on 100% of CIP groove welds subjected to transversely applied tension loading in but, T-and corner joints, in materials 5/16-inch thick or greater. Testing rate must be increased if > 5% of welds have unacceptable defects. LO-1, AISC 341-10): All fabricated steel and their connections shall be inspected to verify compliance with the details shown in the approved plans. Shall be on the premises during the placement of anchor rods/embedments. Verify diameter, grade, type, and length of element and the extent or depth of
Access holes (flange > 2") Welded Joints subject to fatigue Other Steel Inspections (Table N5.) Structural steel details Anchor rods/embeds supporting structural steel	+	•	increased if > 5% of welds have unacceptable defects. Ultrasonic testing shall be performed on 100% of CJP groove welds subjected to transversely applied tension loading in butt, T-and corner joints, in materials 5/16-inch thick or greater. Testing rate must be increased if > 5% of welds have unacceptable defects. LO-1, AISC 341-10): All fabricated steel and their connections shall be inspected to verify compliance with the details shown in the approved plans. Shall be on the premises during the placement of anchor rods/embedments. Verify diameter, grade, type, and length of element and the extent or depth of embedment prior to placement of concrete. Verify contour and finish as well as dimensional tolerances (see Table J8-1 of AISC 341).
Access holes (flange > 2") Welded Joints subject to fatigue Other Steel Inspections (Table N5.: Structural steel details Anchor rods/embeds supporting structural steel Reduced beam sections (RBS)	• 7, AISC 360-16; Tak	· · ·	Ultrasonic testing shall be performed on 100% of CJP groove welds subjected to transversely applied tension loading in butt, T-and corner joints, in materials 5/16-inch thick or greater. Testing rate must be increased if > 5% of welds have unacceptable defects. 10-1, AISC 341-10): All fabricated steel and their connections shall be inspected to verify compliance with the details shown in the approved plans. Shall be on the premises during the placement of anchor rods/embedments. Verify diameter, grade, type, and length of element and the extent or depth of embedment prior to placement of concrete. Verify contour and finish as well as dimensional tolerances (see Table J8-1 of AISC 341). Verify that no holes or unapproved attachments are made within the protected zone (see Table J8-1 of AISC 341)
Access holes (flange > 2") Welded Joints subject to fatigue Other Steel Inspections (Table N5.) Structural steel details Anchor rods/embeds supporting structural steel Reduced beam sections (RBS) Protected Zones OPEN-WEB STEEL JOIST AND JOIST	+ 7, AISC 360-16; Tab	• • • • C Section 1705	Ultrasonic testing shall be performed on 100% of CJP groove welds subjected to transversely applied tension loading in butt, T-and corner joints, in materials 5/16-inch thick or greater. Testing rate must be increased if > 5% of welds have unacceptable defects. 10-1, AISC 341-10): All fabricated steel and their connections shall be inspected to verify compliance with the details shown in the approved plans. Shall be on the premises during the placement of anchor rods/embedments. Verify diameter, grade, type, and length of element and the extent or depth of embedment prior to placement of concrete. Verify contour and finish as well as dimensional tolerances (see Table J8-1 of AISC 341). Verify that no holes or unapproved attachments are made within the protected zone (see Table J8-1 of AISC 341)
Access holes (flange > 2") Welded Joints subject to fatigue Other Steel Inspections (Table N5.) Structural steel details Anchor rods/embeds supporting structural steel Reduced beam sections (RBS) Protected Zones OPEN-WEB STEEL JOIST AND JOIST Installation of Open-Web Steel Jois	+ 7, AISC 360-16; Tab	C Section 1705	Ultrasonic testing shall be performed on 100% of CJP groove welds subjected to transversely applied tension loading in butt, T-and corner joints, in materials 5/16-inch thick or greater. Testing rate must be increased if > 5% of welds have unacceptable defects. 10-1, AISC 341-10): All fabricated steel and their connections shall be inspected to verify compliance with the details shown in the approved plans. Shall be on the premises during the placement of anchor rods/embedments. Verify diameter, grade, type, and length of element and the extent or depth of embedment prior to placement of concrete. Verify contour and finish as well as dimensional tolerances (see Table J8-1 of AISC 341). Verify that no holes or unapproved attachments are made within the protected zone (see Table J8-1 of AISC 341) 1.2.3 & TABLE 1705.2.3)
Access holes (flange > 2") Welded Joints subject to fatigue Other Steel Inspections (Table N5.) Structural steel details Anchor rods/embeds supporting structural steel Reduced beam sections (RBS) Protected Zones OPEN-WEB STEEL JOIST AND JOIST Installation of Open-Web Steel Jois End connections – welding or bolted.	+ 7, AISC 360-16; Tab	• • • • C Section 1705	Ultrasonic testing shall be performed on 100% of CJP groove welds subjected to transversely applied tension loading in butt, T-and corner joints, in materials 5/16-inch thick or greater. Testing rate must be increased if > 5% of welds have unacceptable defects. 10-1, AISC 341-10): All fabricated steel and their connections shall be inspected to verify compliance with the details shown in the approved plans. Shall be on the premises during the placement of anchor rods/embedments. Verify diameter, grade, type, and length of element and the extent or depth of embedment prior to placement of concrete. Verify contour and finish as well as dimensional tolerances (see Table J8-1 of AISC 341). Verify that no holes or unapproved attachments are made within the protected zone (see Table J8-1 of AISC 341)
Access holes (flange > 2") Welded Joints subject to fatigue Other Steel Inspections (Table N5.) Structural steel details Anchor rods/embeds supporting structural steel Reduced beam sections (RBS) Protected Zones OPEN-WEB STEEL JOIST AND JOIST Installation of Open-Web Steel Jois	+ 7, AISC 360-16; Tab	C Section 1705	Ultrasonic testing shall be performed on 100% of CJP groove welds subjected to transversely applied tension loading in butt, T-and corner joints, in materials 5/16-inch thick or greater. Testing rate must be increased if > 5% of welds have unacceptable defects. 10-1, AISC 341-10): All fabricated steel and their connections shall be inspected to verify compliance with the details shown in the approved plans. Shall be on the premises during the placement of anchor rods/embedments. Verify diameter, grade, type, and length of element and the extent or depth of embedment prior to placement of concrete. Verify contour and finish as well as dimensional tolerances (see Table J8-1 of AISC 341). Verify that no holes or unapproved attachments are made within the protected zone (see Table J8-1 of AISC 341) 1.2.3 & TABLE 1705.2.3)





PLAN NOTES:

(1) ALL PAD FOOTINGS ARE CENTERED BELOW THE COLUMN THEY SUPPORT -SEE FOUNDATION PLAN & PAD FOOTING SCHEDULE FOR SIZE & REINFORCING.

(2) ALL CONTINUOUS WALL FOOTINGS ARE 12" THICK UNREINFORCED, AND PROJECT 6" BEYOND EACH FACE OF FOUNDATION THEY SUPPORT, EXCEPT AS NOTED OTHERWISE.

(3) S - INDICATES STEP IN CONTINUOUS WALL FOOTING, SPACE STEPS AT 4'-0" FROM BUILDING CORNERS AND OTHER FOOTINGS STEPS EXCEPT AS SHOWN OTHERWISE.

(4) $\frac{X'-X''}{T.O.F.}$ INDICATES TOP OF FOOTING ELEVATION.

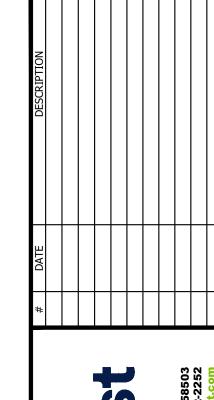
(X) $\stackrel{ ext{$\langle \times \rangle$}}{}$ Indicates Pier Detail- Refer to Pier Details located on sheet 5302 FOR PIER SIZE & REINFORCING.

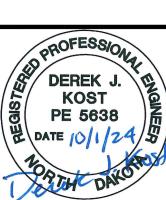
(5) "EX." & "EXIST'G" INDICATES EXISTING CONSTRUCTION. EXIST'G MEMBER SIZES & DIMENSIONS ARE ASSUMED- FIELD VERIFY AS REQ'D.

(6) "F.V." INDICATES THAT DIMENSION IS TO BE FIELD VERIFIED PRIOR TO MATERIAL FABRICATION, CONCRETE CONSTRUCTION, OR STEEL FABRICATION

(7) GENERAL CONTRACTOR TO COORDINATE WITH ELECTRICAL CONTRACTOR FOR BUILDING GROUNDING WIRE ATTACHMENT TO FOUNDATION WALL REINFORGING PRIOR TO POURING FOUNDATION WALL.

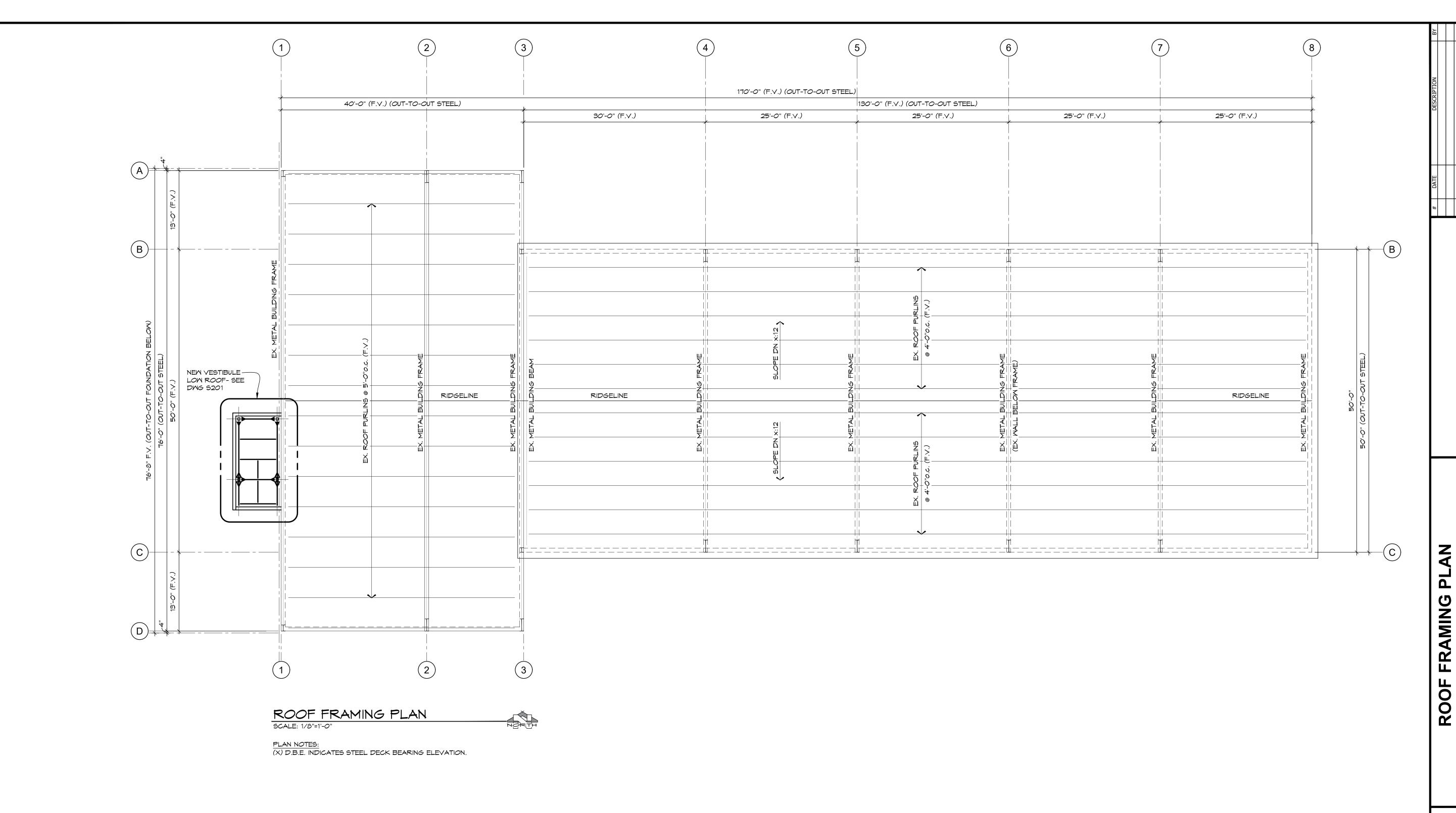






DRAWN BY: RWB / DJK APPROVED BY: DJK DESIGN PROJ: 12493.700 CWS PROJ: C2379A AS NOTED OCT. 2024 DRAWING NO:

S101





DEREK J.

KOST
PE 5638

DATE 10/1/24

DESIGNED BY:

DIK

DRAWN BY:

RWB / DJK

APPROVED BY:

DJK

DESIGN PROJ:

CWS PROJ:

CWS PROJ:

CALE:

AS NOTED

DATE:

DESIGNED BY: DJK

DRAWN BY: RWB / DJK

APPROVED BY: DJK

DESIGN PROJ: 12493.700

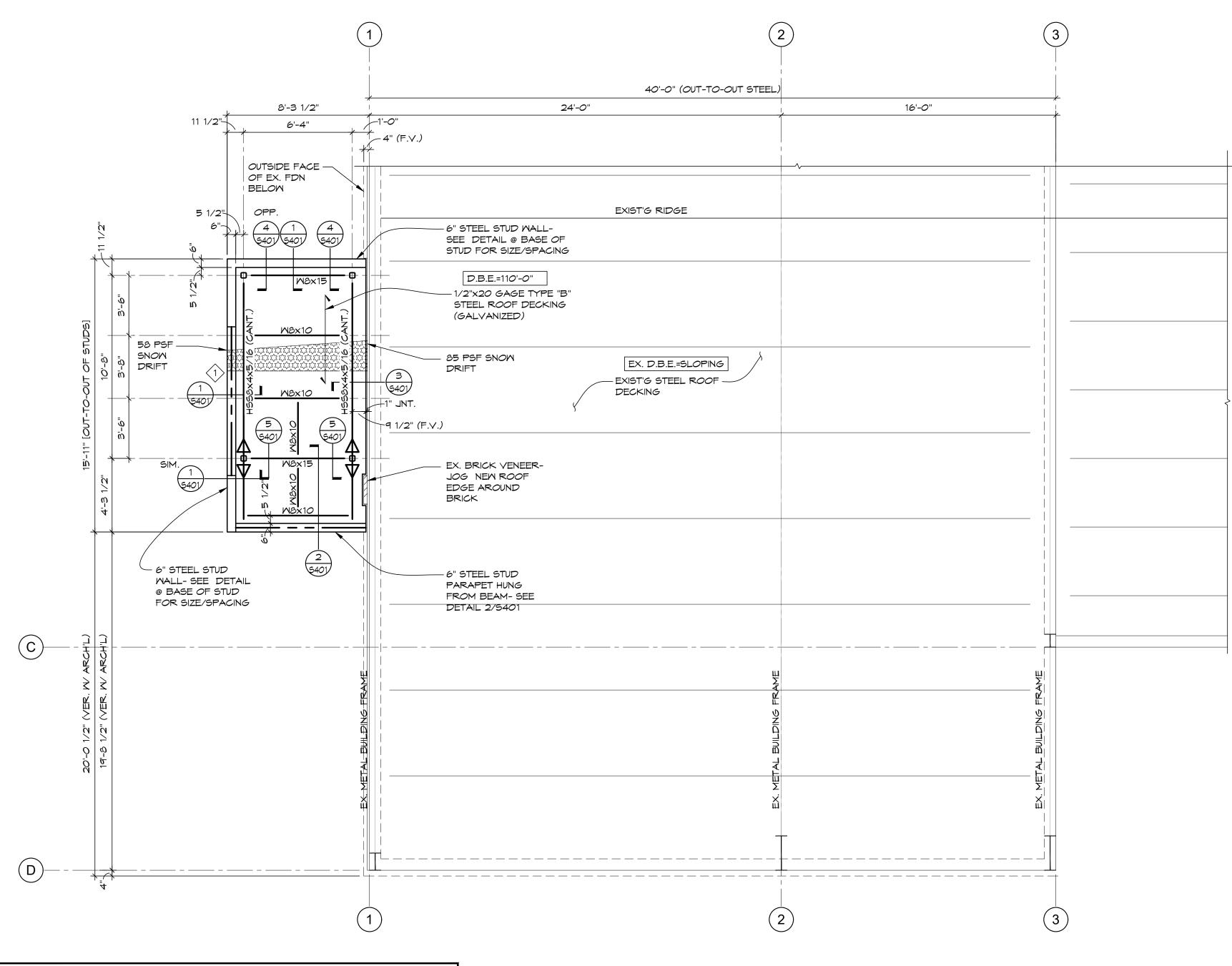
CWS PROJ: C2379A

SCALE: AS NOTED

DATE: OCT. 2024

DRAWING NO:

S201



COLD-FORMED HEADER SCHEDULE								
MARK	HEADER		JAMB		SILL			
MARK	SIZE	CONN.	SIZE	CONN.	SIZE	CONN.		
	600QFH300-54	10/5401	600QFJ300-54	9/5401 12/5601	600T200-54	11/5401		
STEEL STUD HEADER NOTES: (1) ALL HEADERS AND JAMB STUDS ARE TO BE MARINO WARE QUICKERAME MEMBERS								

(OR APPROVED EQUAL).

VESTIBULE ROOF FRAMING PLAN
SCALE: 1/4"=1'-0"

(1) D.B.E. INDICATES STEEL DECK BEARING ELEVATION. ALL NEW ENTRY STEEL BEAMS SHOWN ARE AT T.O.S. = D.B.E. UNLESS OTHERWISE NOTED.

(2) NDICATES COLD FORMED HEADER- SEE PLAN & COLD FORMED HEADER SCHEDULE FOR HEADER SIZE & JAMB STUDS REQUIRED AT WALL OPENING.



AS NOTED OCT. 2024 **S202**

DRAWN BY:

APPROVED BY:

DESIGN PROJ:

CWS PROJ:

RWB / DJK

12493.700

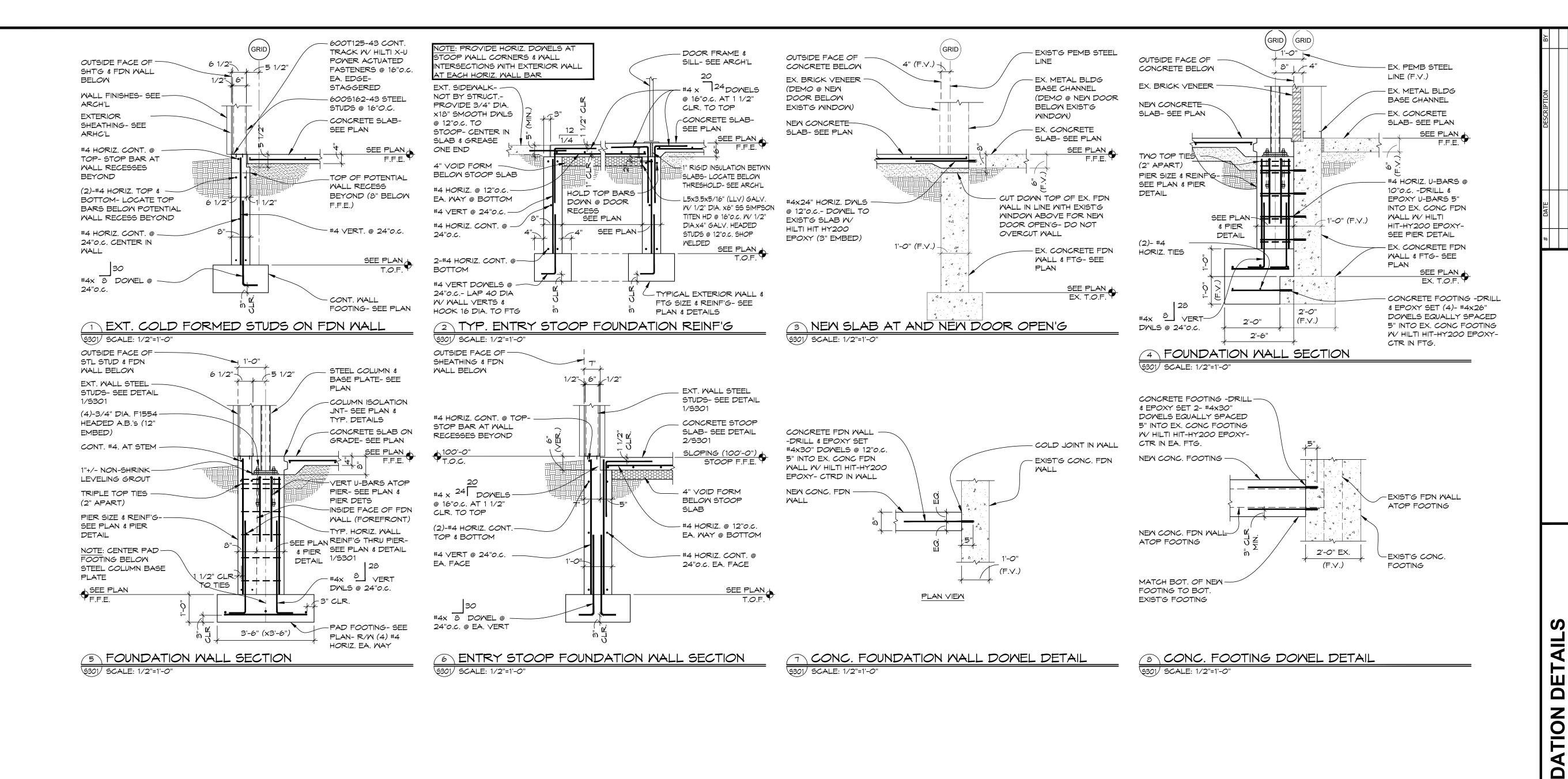
C2379A

DJK

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AMING ROOF

M



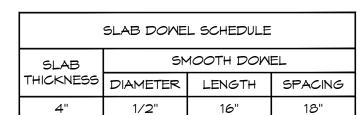


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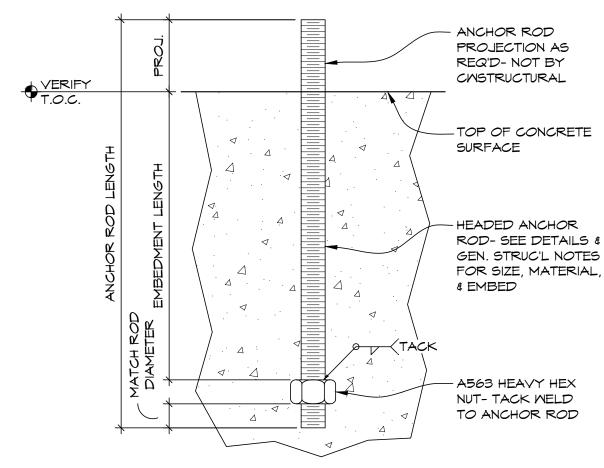
9

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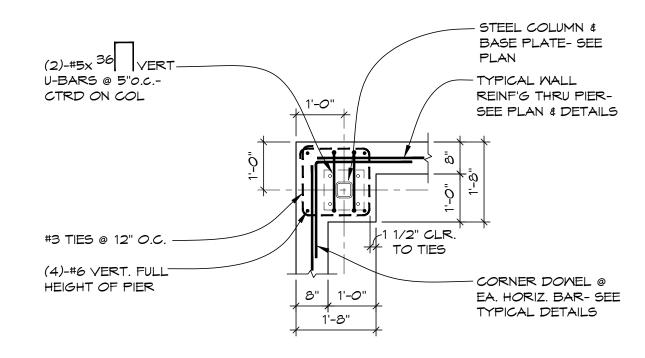
S301



1 TYPICAL SLAB-ON-GRADE JOINTING DETAIL 5302/ SCALE: NO SCALE

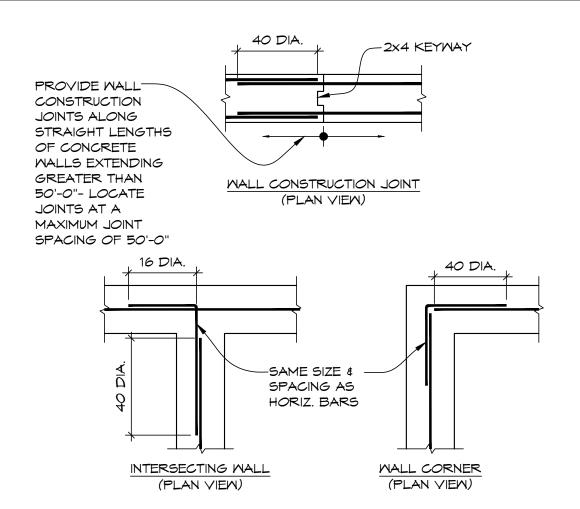


5 TYPICAL HEADED ANCHOR ROD DETAIL 6302 SCALE: NONE

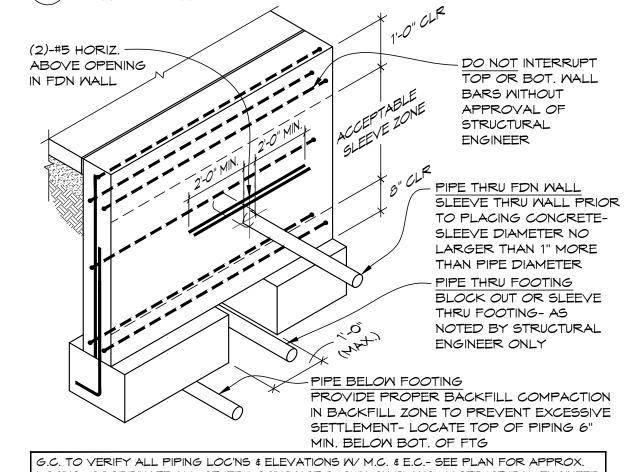


A PIER REINFORCING DETAIL

SCALE: 1/2"=1'-0"



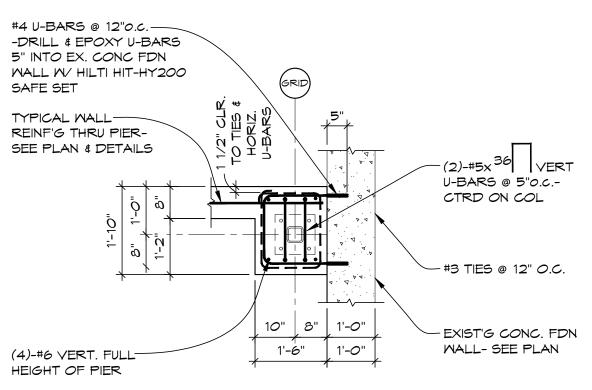
2 TYP. CONCRETE WALL REINFORCING DETS 5302 SCALE: NO SCALE



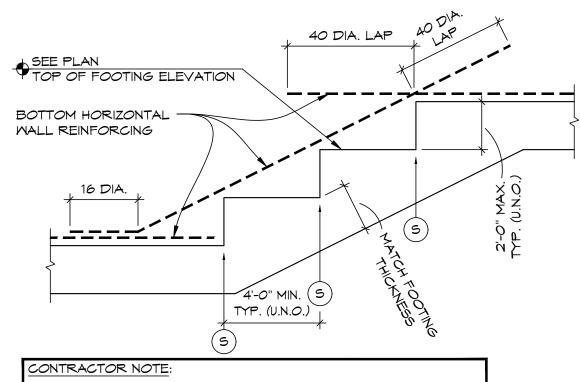
LOCINS- COORDINATE ALL OTHER LOCINS NOT SHOWN ON PLANS W/ STRUCTURAL ENGINEER

6 TYP. PIPING REQUIREMENTS CROSSING FDNS

5302/ SCALE: 1/2"=1'-0"







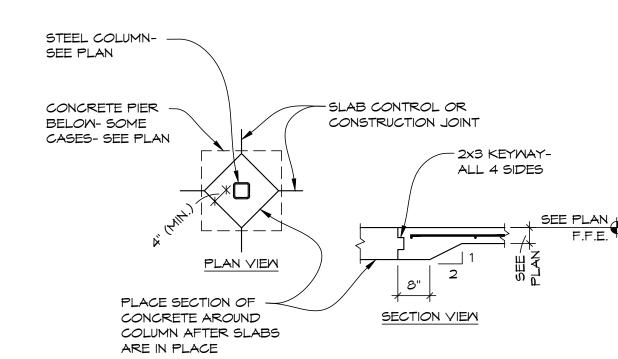
FOOTING STEP LOCATIONS & SPACINGS

3 TYPICAL FOOTING STEP DETAIL

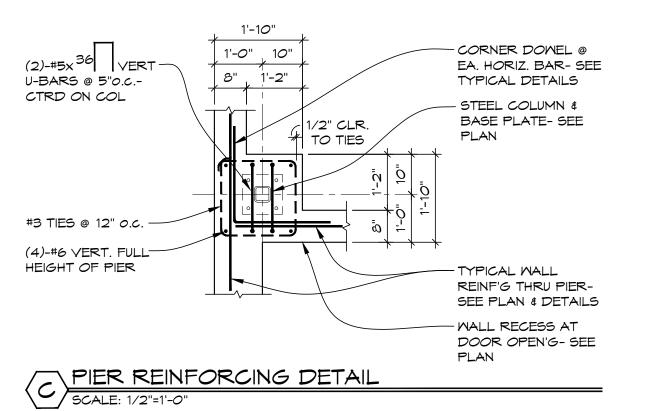
5302 SCALE: NO SCALE

"(S)" INDICATES A STEP IN THE TOP OF CONCRETE FOOTING

ELEVATION- REFER TO THE FOUNDATION PLAN & NOTES FOR



4 TYPICAL COLUMN ISOLATION JOINT DETAIL 5302/ SCALE: NO SCALE





ENGINEERS

1000 E. CALGARY AVE, SUITE 2

BISMARCK, NORTH DAKOTA 58503

701.221.3286

WWW.CWSTRUCTURAL.NET

TYP. FOUNDATION DETA & PIER DETAILS

MR&I OF W CAUSEWAY RD

DESIGNED BY:
DJK
DRAWN BY:
RWB / DJK
APPROVED BY:
DJK
DESIGN PROJ:
12493.700
CWS PROJ:
C2379A
SCALE:
AS NOTED
DATE:
OCT. 2024

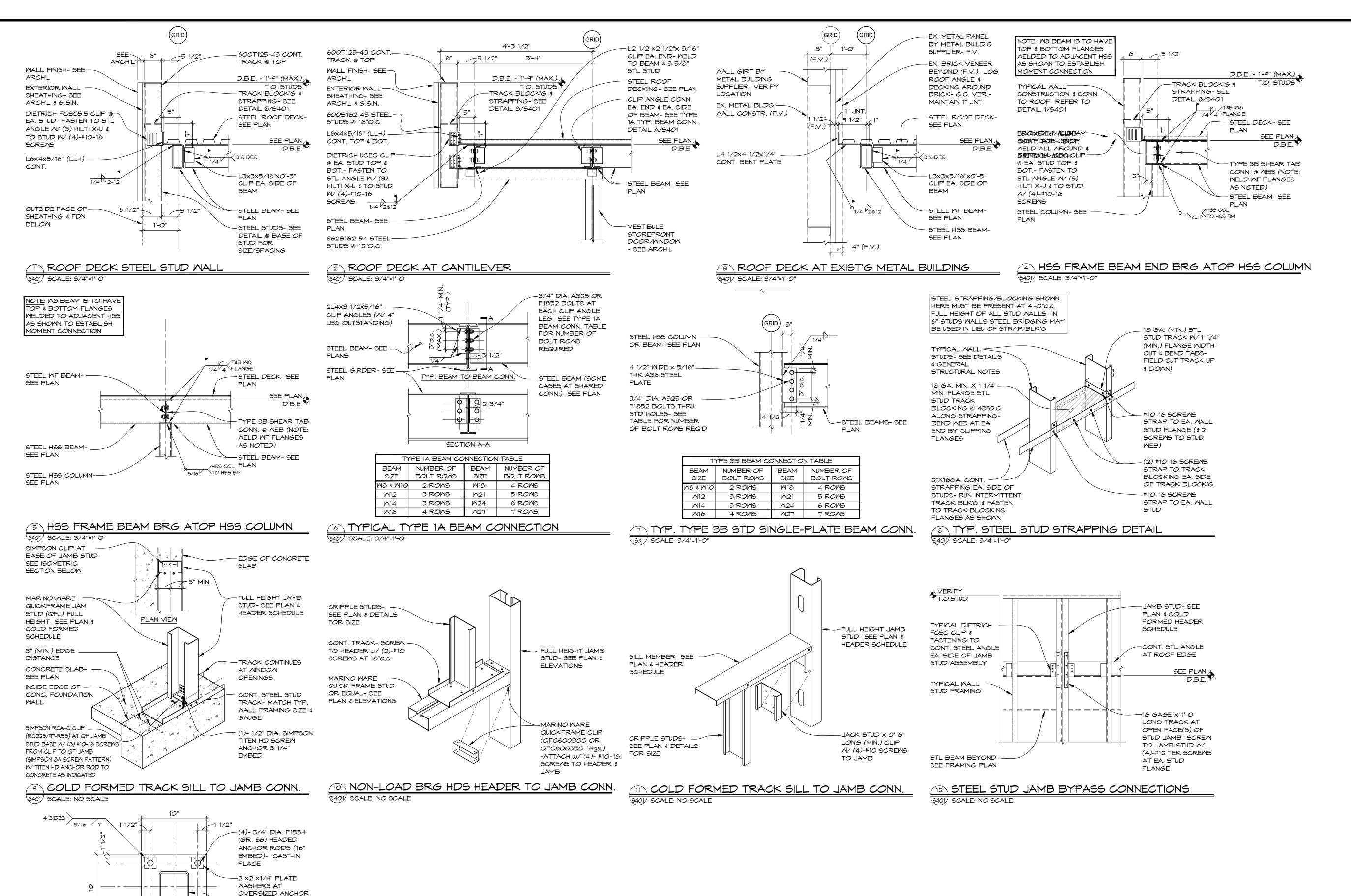
DEREK J. KOST PE 5638

DATE: OCT. 2024

DRAWING NO:

\$302

SHEET NO:



ROD HOLE IN BASE

-STEEL COLUMN- SEE

10"x10"x3/4" THICK

PLATE PER AISC

TABLE 14-2

BASE PLATE

PLAN

TYPICAL HSS4x4 COLUMN BASE PLATE

13 TYPICAL "TYPE 1" COLUMN BASE PLATE

5401/ SCALE: 3/4"=1'-0"



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DESIGNED BY:
DJK
DRAWN BY:
RWB / DJK
APPROVED BY:
DJK
DESIGN PROJ:
12493.700
CWS PROJ:
C2379A
SCALE:
AS NOTED
DATE:
OCT. 2024
DRAWING NO:
S401

SHEET NO:

QOFESS/O

DEREK J.

KOST PE 5638

DE.

FRAMING

DEI ES,

GENERAL DESIGN AND CONSTRUCTION:

- (1) All work shall comply with the 2021 International Building Code (IBC 2021).
- (2) Design Loads:

Project Location: Fort Yates, ND

Wind loads: Per ASCE 7, Exposure Category "C", lw = 1.0 Basic wind speed (Mapped 3-second gust) = 112 mph (Risk Category II)

Snow Load: Per ASCE 7 [Also see plan for potential additional snow drifting] Calculated Roof Snow, Pf = 35 psf Ground Snow (Mapped), Pg = 50 psf, Is = 1.0, Ce = 1.0, Ct = 1.0

Seismic loads per ASCE 7 (Fort Yates, ND) Site class = D (Assumed), Ss = 0.06, S1 = 0.024, le=1.0 Site coefficient, Fa = 1.6, Site coefficient, Fv = 2.4 Seismic Design Category A

Roof Dead Load: Roof Structure (Framing & Decking) Self Weight = 10 psf Roof Collateral Dead Load = 10 psf

- (3) Specific notes and details shall take precedence over General Structural Notes.
- (4) The contract structural drawings and specifications represent the finished structure. Unless otherwise indicated, they do not indicate the means or method of construction. The contractor is solely responsible for the protection of the structure during all phases of demolition, construction and installation. Provide all measures necessary to protect the structure, workers or other persons by means of shoring, bracing and job site safety measures.
- (5) Means-and-methods including temporary bracing and shoring against wind and erection is the responsibility of the contractor.
- (6) No area of the structure shall be loaded with construction material or equipment that exceeds final design loading indicated.
- (7) Verify location of box-outs and openings with mechanical and electrical contractors. Opening sizes and locations shown for pipes, ducts, etc. are for general information only and shall be verified with the mechanical and electrical contractors before commencing with work.
- (8) Holes, pipes, sleeves, etc. through structural framing and foundations that are not shown on the drawings are not acceptable.
- (9) Structural engineer's seal on the plan does not provide for construction inspection.
- (10) The cost for additional structural engineering services necessitated by contractor requests for an option or due to errors or omissions in construction shall be the contractor's responsibility.
- (11) Shop drawings prepared by suppliers, subcontractors, etc. shall be dimensioned, reviewed, coordinated, and signed/stamped by the general contractor prior to submitting to the structural engineer. Manufactured components such as steel stairs, trusses or precast concrete shall be engineered and stamped by a licensed Professional Engineer in the state the project is being built prior to submission.
- (12) Verify all dimensions and conditions of existing construction to be as shown on the drawings. Advise the Architect and Structural Engineer of variances prior to continuing with construction.
- (13) Protect existing construction from damage due to construction of new additions. Make no cuts or alterations to existing construction other than those shown on the drawings without the approval of the Architect and Structural Engineer.
- (14) All necessary egress exit/entry doors must be constructed and maintained to operate freely at all times providing egress from the building as well as satisfying and maintaining any and all ADA requirements. Design and construction of exterior concrete sidewalks, stoops, paving, door sills, etc., to provide and maintain egress in a frost-free manner via stoop foundations, non-frost susceptible fill, flowable fill, shallow insulated frost-protected foundation or other means is not the responsibility of CWSTRUCTURAL Engineers.
- (15) CWSTRUCTURAL may provide periodic observations to assure conformance with design intent of the construction documents. However, these observations are not meant to fulfill the requirements of the IBC required special inspections. CWSTRUCTURAL is not considered a qualified "Special Inspector" as it relates to required building code. Refer to the Special Inspections.

STATEMENT OF SPECIAL INSPECTIONS:

(1) Special inspections and structural testing shall be provided by an independent agency employed by the Owner, unless specified differently in project specifications, for the items identified in this section and in other areas of the approved construction plans and specifications, unless waived by the Building Official (see IBC Chapter 17).

- (2) The names and credentials of the Special Inspectors to be used shall be submitted to the Building Official for approval.
- (3) Duties of the Special Inspector:
- a. The Special Inspector shall review all work listed below for conformance with the approved construction plans and
- specifications and the IBC. b. The Special Inspector shall furnish special inspection reports to the EOR, Contractor, Owner and Building Official on a weekly basis, or more frequently as required by the Building Official. All items not in compliance shall be brought to the immediate attention of the Contractor for correction, and if uncorrected, to the EOR and the Building Official.
- c. Once corrections have been made by the Contractor, the Special Inspector shall submit a final signed report to the Building Official stating that the work requiring special inspection was, to the best of the Special Inspector's knowledge, in conformance with the approved construction plans and specifications as well as the applicable workmanship provisions of the IBC.
- (1) Duties and responsibilities of the Contractor:
 - a. The Contractor shall submit a written statement of responsibility to the Owner and the Building Official prior to the commencement of work. In accordance with IBC 1704.4, the statement of responsibility shall contain acknowledgement of the special inspection requirements contained within the "Statement of Special Inspections".
- b. The Contractor shall notify the responsible Special Inspector that work is ready for inspection at least one work day (24 hours minimum) before such inspection is required.
- c. All work requiring special inspection shall remain accessible and exposed until it has been observed by the Special

(1) REQUIRED SPECIAL INSPECTIONS (Per IBC 2021):

CONCRETE (IBC Section 1705.3): Special inspections to be performed per "Required Special Inspection of Concrete Construction" table located within these construction documents and also called out in the IBC table 1705.3. (Exception: Special inspections are not required for concrete in the following cases:

- 1) Isolated spread concrete footings of buildings three stories of less above grade plane that are fully supported on earth or
- 2) Continuous concrete footings supporting walls of buildings three stories or less above grade plane that are fully supported on
- earth of rock where: 1.1) The footings supporting walls of light-framed construction
- 1.2) The footings are designed in accordance with Table 1809.7 of the IBC.
- 1.3) The structural design of the footing is based on a specified compressive strength, f'c, not more than 2,500psi, regardless of the compressive strength specified in the approved construction documents or used in the footing construction.
- 3) Nonstructural concrete slabs supported directly on the ground, including prestressed slabs on grade, where the effective
- prestress in the concrete is less than 150 psi. 4) Concrete foundation walls constructed in accordance with Table 1807.1.6.2.
- 5) Concrete patios, driveways and sidewalks, on grade.)
- WIND-RESISTING COMPONENTS (IBC Section 1705.11.3): Periodic special inspection is required for fastening of the following 1.) Roof coverings, roof deck, and roof framing connections.
- 2.) Exterior wall covering, and wall connections to roof and floor diaphragms and framing.(i.e. Wood Studs, Steel Studs, CMU

SOIL (IBC Section 1705.6): Special inspections to be performed per Table 1705.6 located both in the IBC and within these

construction documents

STRUCTURAL STEEL & STEEL JOISTS (IBC Section 1705.2, 1705.11, & 1705.12, and AISC 360): Special inspections to be performed per "Required Special Inspection of Structural Steel" table located within these construction documents and also called out in the IBC & AISC.

(6) Please see the "Special Inspection Schedules/Tables" for the types, extents and frequency of specific items requiring special inspections and structural tests as part of this project. Inspections and testing indicated in the special inspection tables are guidelines to the required inspection and testing required by the building code for the particular building. The IBC and referenced testing/inspection standards shall be reviewed and followed by the inspecting agency to ensure that all required testing/inspection and procedure for testing/inspection is achieved.

FOUNDATIONS:

(1) Assumed Geotechnical Design Values - All concrete foundation designs are based on assumed geotechnical design values to be confirmed by a geotechnical engineer. All foundation excavations must be observed and all assumed design values stated below shall be verified in writing by a licensed geotechnical engineer following on-site observation prior to fabricating steel and placing concrete any concrete. All assumed geotechnical design values are as follows:

A.Minimum Soil Bearing Capacity = 1500 psf B. Backfill Soil Unit Weight (γ) = 110 pcf (dry)

- (1) All footings shall bear on natural, undisturbed soil. All natural bearing material shall be approved in writing by a licensed Geotechnical Engineer prior to placing footing concrete.
- (2) All seepage shall be continuously pumped from excavations until the Geotechnical Engineer of Record determines such seepage no longer impacts the bearing soils or Engineered Fill or construction of footings and floor slabs.
- (3) Positive drainage shall be obtained away from the structures and care required for backfilling and drainage of utility trenches during construction shall be provided per the Geotechnical Report and the Civil drawings.
- (4) Excavations shall be performed in accordance with all governing safety regulations including OSHA. There shall be no surcharge load from vehicles, equipment, materials, soil piles etc. near the crest of the excavation slopes per the Geotechnical Report. The responsibility for excavation safety and temporary construction slopes lies solely with the Contractor.
- (5) A 6" thick aggregate base course acting as a capillary break and a vapor barrier shall be placed below all interior slabs-on-grade per the Geotechnical Report and the project specifications. A minimum 15 mil vapor barrier shall be placed on top of the aggregate base and directly below the slab with all seams overlapped a minimum of 6" and taped. The slab shall be properly cured as indicated in these General Structural Notes and the project specifications as well as per the recommendations of the
- (6) Clean footing excavations of snow, water, mud, loose soil and debris prior to placing footing concrete.
- (7) Footings may not be earth formed.
- (8) All footings are centered under the steel columns or walls they support, except as noted otherwise.
- (9) Excavations and backfill shall be executed and tested in accordance with the project specifications and soils report.
- (10) Footing excavations shall be to proper line and level to insure minimum concrete cover of footing reinforcement for footing depth.
- (11) Backfill shall be compacted by mechanical means.
- (12) Place all backfill according to the project specifications. General contractor is required to temporarily brace all walls as required prior to and during the placing of backfill and until permanent support for the top and base of the walls are completely constructed.
- (13) Protect all foundations from the action of water and freezing.
- (14) See mechanical, electrical and architectural drawings for all openings and inserts not shown on the structural drawings. All openings and inserts shall be placed prior to casting concrete.

CONCRETE:

- (1) Concrete mix design(s) shall be by an independent testing laboratory and shall be submitted to the Architect and Structural Engineer for approval at the responsibility of the General Contractor.
- (2) Concrete Mix Designs:

Footings are to have a concrete mix design consisting of Portland Type I/II cement (5 sack min.) and a 28-day compressive strength of 4500 psi, 25% max fly ash allowed, 3/4" max aggregate size, 4" max slump, 0.50 max water-to-cement ratio, no air entrainment, water reducing admixture ok

Foundation Walls are to have a concrete mix design consisting of Portland Type I/II cement (6 sack min.) and a 28-day compressive strength of 4500 psi, 25% max fly ash allowed, 3/4" max aggregate size, 4" max slump, 0.45 max water-to-cement ratio, 5% to 7% air entrainment, water reducing admixture ok, mid range plasticizer ok, super plasticizer requires prior approval.

Interior Slabs-on-Grade are to have a concrete mix design consisting of Portland Type I/II cement (5 sack min.) and a 28-day compressive strength of 4500 psi, 25% max fly ash (Note: fly ash will retard initial set time), 3/4" max aggregate size, 4" max slump, 0.48 max water-to-cement ratio, no air entrainment, water reducing admixture ok, mid range and super plasticizer requires

Exterior exposed elevated slabs (Stoop slabs) are to have a concrete mix design consisting of Portland Type I/II cement (6 sack min.) and a 28-day compressive strength of 4,500 psi, 30% max fly ash (Note: fly ash will retard initial set time), 3/4" max aggregate size, 3" max slump, 0.41 max water-to-cement ratio, 5% to 7% air entrainment, water reducing admixture ok, mid range ok, super plasticizer requires prior approval.

Grout for steel base plates and bearing plates is to be of the non-shrink non-metallic type meeting ASTM C1107 with a maximum slump of 3" and a minimum 28-day compressive strength of 5000 psi. Follow manufacturer's directions for use.

- (3) Concrete aggregate shall meet ASTM C33 with a maximum shale or deleterious material content of 1%.
- (4) Portland cement shall meet ASTM C150 and shall be low Alkali.
- (5) Fly Ash of "Type C meeting Class F" may be substituted by weight for cement up to maximum limits indicated in each concrete
- (6) Mid-Range Plasticizer meeting ASTM C494 Type D is acceptable if noted in mix design. Mix is to have a maximum slump as indicated above for the particular mix design prior to adding the Mid-Range. Adjust air content as required by the supplier due to the use of the Mid-Range and its projected effect on the air content. Test for air after Mid-Range addition to achieve range specified. Slump is to be tested to meet that specified above for particular mix design prior to adding Mid-Range Plasticizer and no additional water may be added after slump test.
- (7) Air Entraining agents shall meet ASTM C260.
- (8) Concrete construction shall conform to the ACI building code requirements for reinforced concrete, ACI 318.
- (9) Hot Weather Concreting per ACI 305R and Cold Weather Placement per ACI 306R shall be followed where weather conditions
- (10) Slabs shall be cured with a curing compound as indicated in the specifications and per the recommendations of the ACI code relative to exposure to sunlight, wind, temperature, etc.
- (11) Forms shall be left on all walls for a minimum of 2 days or longer as required at the discretion of the contractor.
- (12) A continuous bond break, such as 3/8" asphaltic fiber board expansion joint material, shall be placed between the concrete slab and the perimeter foundation walls.
- (13) All concrete pours shall be tested for strength (per ASTM C31 and C39), slump and air content. Test one cylinder at 7 days, one at 14 days, two at 28 days and hold one cylinder.
- (14) All concrete reinforcing shall meet ASTM specification A615, Grade 60.
- (15) Reinforcing steel shall be bent and placed in accordance with the ACI code. All tension splices shall be class 'B'. 48 bar diameters, minimum. All compression splices shall be 40 bar diameters (minimum), unless noted otherwise. Lap all corner bars.

- (16) Provide adequate support bars and accessories to hold all rebar firmly in place.
- (17) All slabs are to be reinforced with bars located as called out on structural drawings. G.C. must use rebar support chairs to hold all slab bars firmly in their required location during pouring. Chairs are to be placed frequently enough to ensure no more than 1/4" of sag is present in any reinforcing bars.
- (18) Concrete cover for reinforcing shall be per ACI 318.
- (19) Slabs-on-grade are to be reinforced as shown on plans and details. Place slabs with construction and contraction joints per
- (20) Openings in concrete shall be reinforced with (2)- #5 bars each side, extending 30" past the face of the opening unless otherwise
- (21) Interior concrete slabs-on-grade must not be allowed to freeze after pouring. Subgrade soil temperature beneath interior slabs-on-grade must be maintained above freezing at all times after pouring. Concrete shall not be poured on frozen soils.
- (22) All reinforcing specified for interior slabs-on-grade and elevated slabs shall be supported adequately and firmly on chairs to maintain the clear distances specified.
- (23) All vertical wall and pier bars must be extended to within 2" of the top of foundation walls unless shown otherwise on the drawings. All horizontal foundation wall bars must be placed within 4" of the top and bottom of the wall unless shown otherwise.
- (24) All epoxy-set rebar doweling to concrete shall be HILTI HIT-HY 200 Safe Set System with HILTI Hollow Drill Bit System, except as noted otherwise. Install per manufacturer's recommendations.

STRUCTURAL STEEL:

- (1) Structural steel work is to be per AISC specifications. Steel grade shall be ASTM A992 (Fy=50 ksi) for wide flange shapes, ASTM A500, Grade B (Fy=46 ksi) for cold-formed steel tubes and ASTM A36 (Fy=36ksi) for other shapes except as noted otherwise on the drawings.
- (2) Beams and columns shall be erected true and plumb. Provide temporary bracing.
- (3) Bearing plates for steel beams and columns shall be dry packed with dry grout as specified.

recommendations with bolt embedment depth into concrete as indicated on structural drawings.

- (4) All bolts in structural steel connections shall be ASTM F3125, Group A, standard heavy hex head or twist-off tension control (TC) High-Strength bolts, except as noted otherwise.
- (5) All anchor rods for structural steel columns shall be ASTM F1554, Grade 36 (Fy=36ksi) threaded rod with a nut on the embedded end as indicated on the drawings.
- (6) All screw anchors shall be carbon steel HILTI KWIK HUS-EZ, except as noted otherwise. Install bolts per manufacturer's
- (7) All epoxy-set threaded rod anchorage to concrete shall be HILTI HIT-HY 200 Safe Set System with carbon-steel anchors (Anchors are to consist of 'HAS-B-105' (Fy=105 ksi) threaded rod material meeting ASTM 193, Grade B7 placed in holes cleaned with the Hilti Hollow Drill Bit System, except as noted otherwise). Install per manufacturer's recommendations with bolt embedment depth into concrete as indicated on structural drawings. Contractor is to provide heat as required to maintain minimum required concrete base material temperature per supplier recommendations during epoxy curing.
- (8) No field welds are to be made until the members are properly aligned. Field welds are to be made by competent certified welders using proper electrodes and amperage.
- (9) Shop welds for 50% of the beam moment splices shown on the details shall be shop inspected and tested by Magnetic Particle Inspection (MT). This testing shall be done on randomly selected connections by an independent testing laboratory with results forwarded to the Architect and Structural Engineer. Joints shall be reworked and tested as necessary. Costs of all testing and retesting shall be borne by the contractor.

STEEL DECKING:

- (1) Metal deck is to be manufactured and erected in accordance with the Steel Deck Institute (SDI) specification.
- (2) 1 1/2" steel roof decking is to have a minimum design yield strength of Fy=33 ksi and shall be fastened with support fasteners of #12 Tek screws in an SDI 36/7 fastener pattern at end members & interior supports. Also provide (1) #10 TEK screw sidelap fasteners at each span, except at the deck lap edge along the roof perimeter where standard support fasteners are to be provided to the perimeter steel supports.
- (3) Steel deck panel ends (other than butted VLI composite decking) must overlap a minimum of 2". End lap and corner lap conditions must be snug and tight to one another and the supporting steel frame prior to frame fastener attachment.
- (4) Powder-actuated deck fasteners may be substituted for indicated Tek screws at supports with approval of structural engineer record. Powder actuated fasteners are to consist of Hilti X-ENP-19 L15 pins for steel thicker than 1/4" and Hilti X-HSN 24 pins for steel between 1/8" and 3/8" thick. Substitutions require approval from the structural engineer. Sidelap fasteners are to be provided as originally indicated at deck sidelaps and at perimeter deck edges.
- (5) 5/8" diameter puddle welds may be substituted for indicated Tek screws at supports decking supports (welding washers are required for decking less than 0.028" thick). Stitch welds per the deck manufacturer's recommendations can be substituted for sidelap screw fasteners, though stitch welds are not to be used for deck less than 0.028" thick. All fastenings shall be made per the steel deck and mechanical fastener manufacturer's recommendations and specifications.
- COLD-FORMED METAL FRAMING (GENERAL):
- (1) Cold-formed framing members are to be provided as indicated on the structural drawings per standard nomenclature established by the American Iron and Steel Institute (AISI) identifying size, type and thickness of each member (studs, tracks, etc.)
- (2) Prior to ordering, fabrication, and installation of cold-formed framing, the contractor shall submit manufacturer's product data of the cold-formed members and clips to the Architect and Structural Engineer for approval.
- (3) Structural framing members shall be properly spaced, plumbed, leveled, squared, fit properly against abutting members and held securely in place until permanently fastened. Wire tying of structural framing members is not permitted.

Punch-outs are to have a maximum width equal to the lesser of half the member depth or 2 1/2", and a maximum length of 4 ½".

(5) Fastening of framing components shall be by screws, powder actuated fasteners, welding or a combination of methods. The type, size, and spacing of fasteners shall be per manufacturer's recommendations, except as noted otherwise on the contract

(4) Factory punch-outs are to be located along the centerline of the member web with a minimum center-to-center spacing of 24"o.c.

- (6) All welded connections shall be made with the latest recommendations of AISI (American Iron and Steel Institute) and the AWS
- (7) Structural framing members having protective coating removed by welding or other fastening method shall have the coating repaired by painting with a zinc-rich primer.
- (8) Structural framing tracks shall have web in contact with a uniform and level bearing surface and be securely anchored as specified in the contract documents or approved connection details.
- (10) WOOD PANEL SHEATHING All exterior wood panel sheathing shall be Exposure 1 rated plywood or OSB sheathing conforming to DOC PS 1 or PS 2 with a 24/16 APA Span Rating. All panels shall be identified with a grade mark of certificate of inspection issued by an APA approved agency. Fasteners are to be installed through the sheathing to the blocking. Sheathing is to be applied perpendicular to framing with sheathing fastened to framing with a minimum No. 8 countersunk tapping screws with a minimum head diameter of 0.285 inch or No. 10, countersunk tapping screws with a minimum head diameter of 0.333 inches. Screws used to attach sheathing are to conform to ASTM C1513 and are to be spaced at 6" o.c. at panel edges (edge spacing) and at 12"o.c. at intermediate framing members (field spacing). Screws are to be located not less than 3/8" from edges of panels & framing.

(9) Temporary bracing of wall framing shall be provided by the contractor and removed only after permanent bracing is installed.

METAL BUILDINGS:

(1) The existing metal building roof panel to be replaced across the entire building is to have its steel thickness (gage) and configuration (profile) verified, along with the existing fastener configuration. All replacement metal building roof panel is to meet or exceed the existing roof panel thickness and configuration for strength and fasteners are to be replaced at the same spacing to ensure existing supporting framing is braced as intended by the original metal building design to support the roof snow loads indicated. Contractor is to coordinate with metal building supplier providing the metal building roof panel.

0 C TRU

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ENER

OFFICE RE RD, FORT

DEREK J. PE 5638

DRAWN BY: RWB / DJK APPROVED BY: DJK DESIGN PROJ: 12493.700 CWS PROJ: C2379A AS NOTED OCT. 2024 DRAWING NO

CWSTRUCTURAL

701.221.3286

ENGINEERS

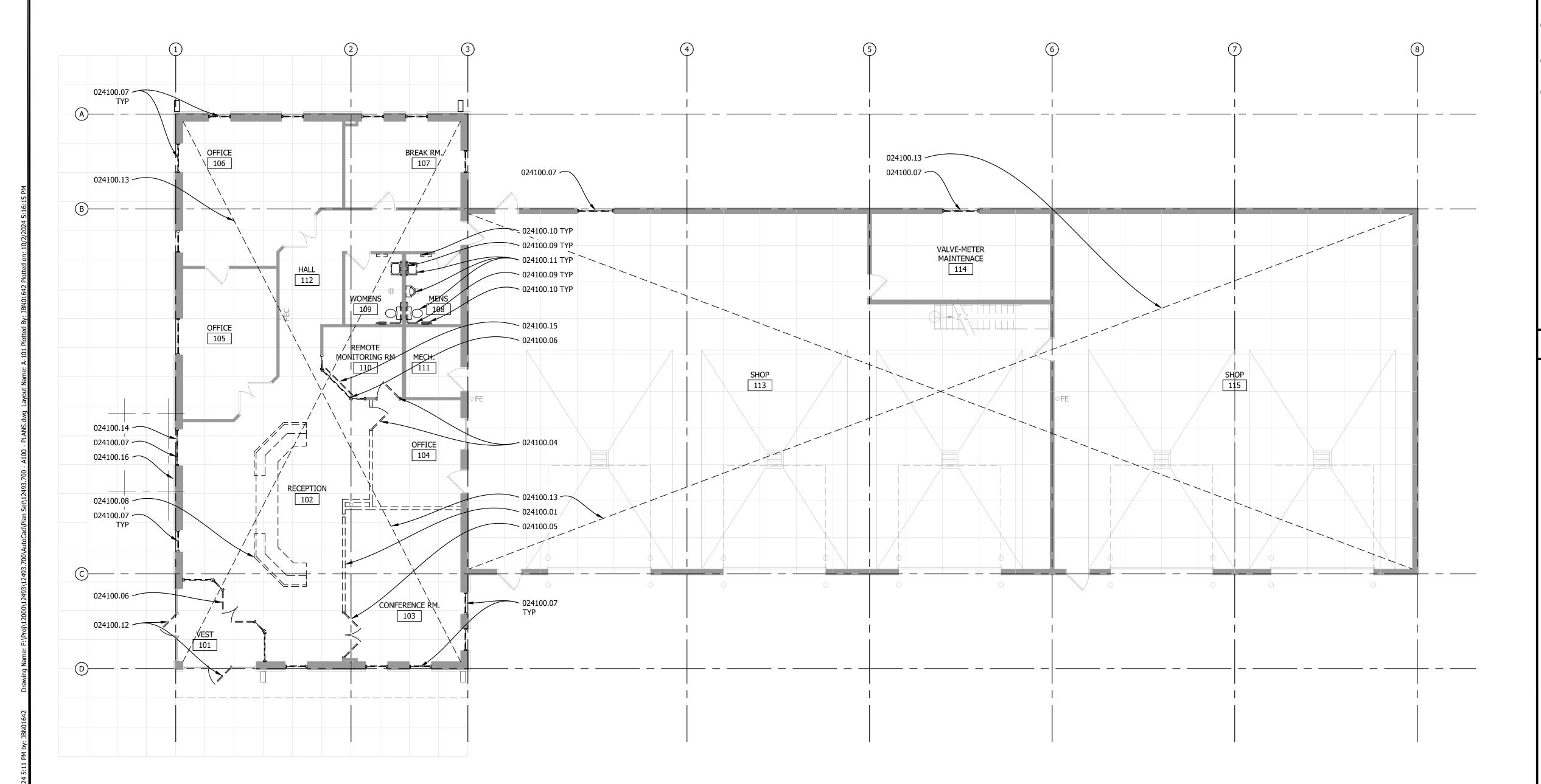
WWW.CWSTRUCTURAL.NET

1000 E. CALGARY AVE, SUITE 2

BISMARCK, NORTH DAKOTA 58503

S501 SHEET NO:

201 | 287 | -- | -- | --



REFERENCE NOTES:

REMOVE AND DISPOSE OF WALL/S AS INDICATED. 024100.01 REMOVE AND DISFOSE OF WILL, EARLY PREP AND CLEAN FLOOR FOR NEW FINISHES.

REMOVE AND DISPOSE OF DOOR AND FRAME 024100.04 ASSEMBLIES AS INDICATED. SALVAGE AND STORE

DOOR WINDOW BLINDS. 024100.05 REMOVE AND S. INDICATED. REMOVE AND SALVAGE DOOR, FRAME, HARDWARE

024100.06 ASSEMBLIES AND RELATED FINISHES. PREP AND CLEAN SURFACES FOR NEW FINISHES. REMOVE AND DISPOSE OF WINDOW SYSTEM AND

REMOVE AND DISPOSE STOREFRONT/GLAZING

024100.07 BLINDS AS INDICATED. PREP OPENING FOR NEW WINDOW. REMOVE AND DISPOSE OF CASEWORK / 024100.08

WORKSURFACE / FURNISHING AS INDICATED.

REMOVE AND DISPOSE OF TOILET ACCESSORIES, 024100.09 MIRRORS, AND RELATED ITEMS. PREP AND CLEAN SURFACES FOR NEW FINISHES.

REMOVE AND DISPOSE OF RECESSED TOILET PAPER HOLDER AND PAPER TOWEL DISPENSOR. PATCH 024100.10 GYPSUM WALL BOARD AND PREPARE FOR NEW WALL FINISHES.

REMOVE AND DISPOSE OF PLUMBING 024100.11 FIXTURE/EQUIPMENT. SEE PLUMBING.

REMOVE AND DISPOSE OF THE STOREFRONT DOOR 024100.12 AND PREPARE FOR NEW DOOR/GLAZING AS INDICATED ON THE PLAN.

REMOVE AND DISPOSE OF METAL ROOFING. INSULATION, AND RELATED FLASHINGS, GUTTERS, DOWNSPOUTS, AND METAL SOFFIT. PREPARE FOR 024100.13 NEW ROOFING AND SOFFIT INSTALLATION. REPORT ANY WATER DAMAGE TO ARCHITECT/ENGINEER

BEFORE COVERING. REMOVE AND DISPOSE OF WALL BELOW EXISTING WINDOW FOR NEW DOOR OPENING. REMOVE BRICK WAINSCOAT AND TOOTH IN AT NEW

OPENING. PREP AND CLEAN FOR NEW FINISHES. REMOVE AND TURN OVER TO OWNER SCADA 024100.15 MONITOR, STAND, AND COMPUTER. SEE ELECTRICAL.

REMOVE AND DISPOSE OF METAL WALL PANELING 024100.16 AS NEEDED TO FACILITATE VESTIBULE INSTALLATION.

GENERAL NOTES:

1. OWNER WILL OCCUPY PORTIONS OF THE BUILDING IMMEDIATELY ADJACENT TO AREAS OF WORK. CONDUCT WORK IN A MANNER THAT WILL MINIMIZE NEED FOR DISRUPTION OF OWNERS OPERATIONS.

2. PROVIDE TEMPORARY BARRIERS TO PROTECT OWNERS PERSONNEL AND GENERAL PUBLIC FROM INJURY DUE TO CONSTRUCTION WORK. TEMPORARY BARRIERS SHOULD KEEP OUT DUST AND CONSTRUCTION DEBRIS FROM OWNER OCCUPIED SPACES.

3. PROTECT FROM DAMAGE EXISTING FINISHES THAT ARE TO REMAIN IN PLACE.

4. CONTRACTOR SHALL VERIFY ALL EXISTING BUILDING DIMENSIONS AND NOTIFY ARCHITECT OF ANY DISCREPANCIES BEFORE START OF WORK.

5. CONTRACTORS TO LOCATE ALL UNDERGROUND UTILITIES PRIOR TO EXCAVATION AND/OR CONSTRUCTION.

6. SEE DEMO SCHEDULE FOR FLOOR AND CEILING DEMOLITION. PREP AND CLEAN ALL SURFACES TO RECEIVE NEW FINISHES.

7. INSPECT ROOFING INSULATION FOR MOLD AND WATER DAMAGE DURING DEMOLITION AND REPORT ANY FOUND DAMAGE TO ARCHITECT BEFORE COVERING.

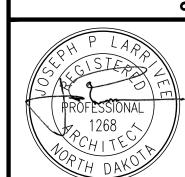
DEMO SCHEDULE ABBREVIATIONS

ACT - REMOVE AND DISPOSE OF EXISTING ACOUSTICAL CEILING TILE AND GRID

GWB CLG - GYPSUM WALL BOARD CEILING CPT - CARPET

PT - PORCELAIN TILE RCB - RUBBER COVE BASE

VCT - VINYL COMPOSITE TILE



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DESIGNED BY:	JPL
DRAWN BY:	JBN
APPROVED BY:	JPL
DESIGN PROJ:	12493.700
CONST PROJ:	
SCALE:	AS NOTED
DATE:	OCTOBER 2024
DRAWING NO:	

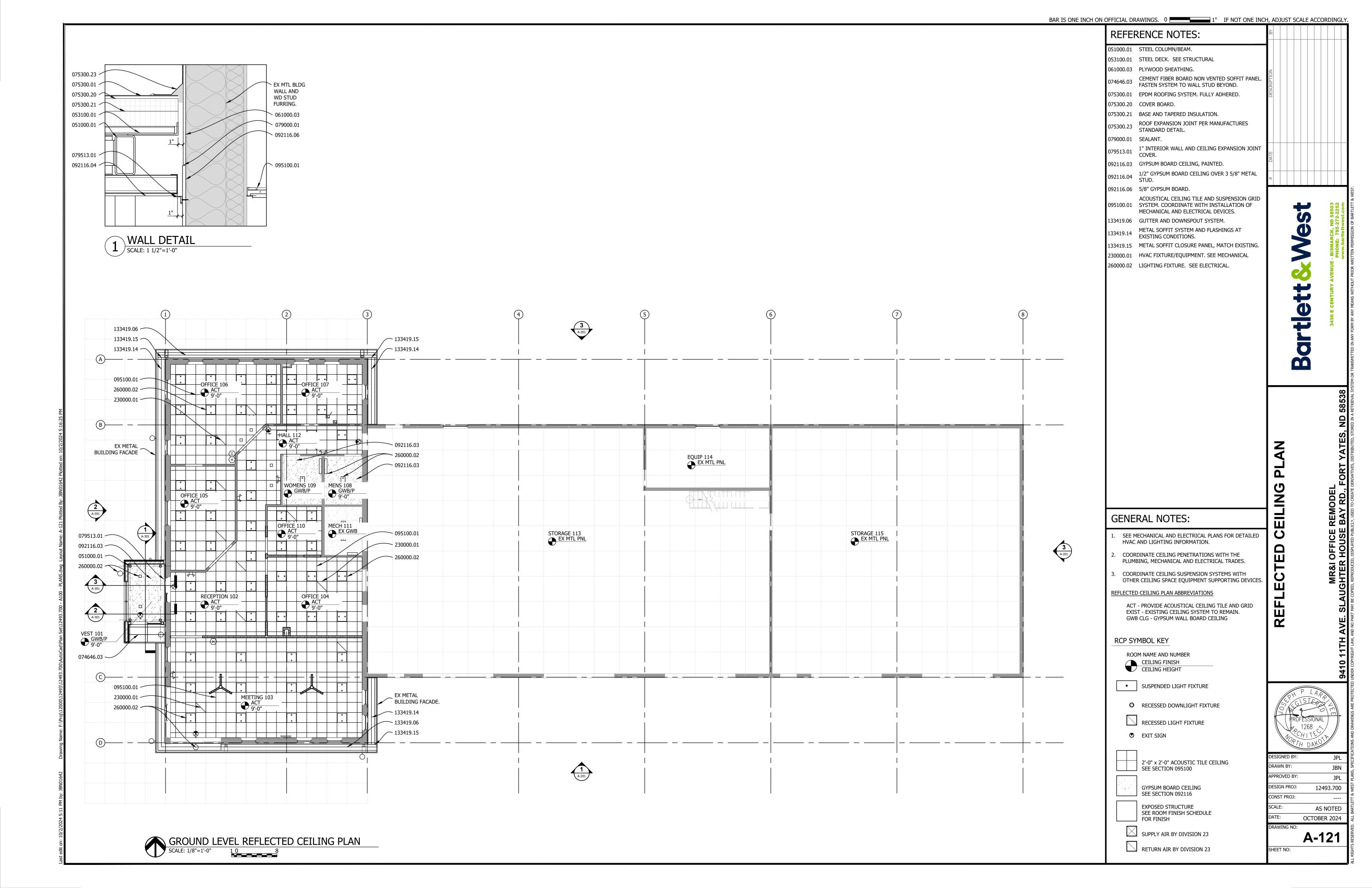
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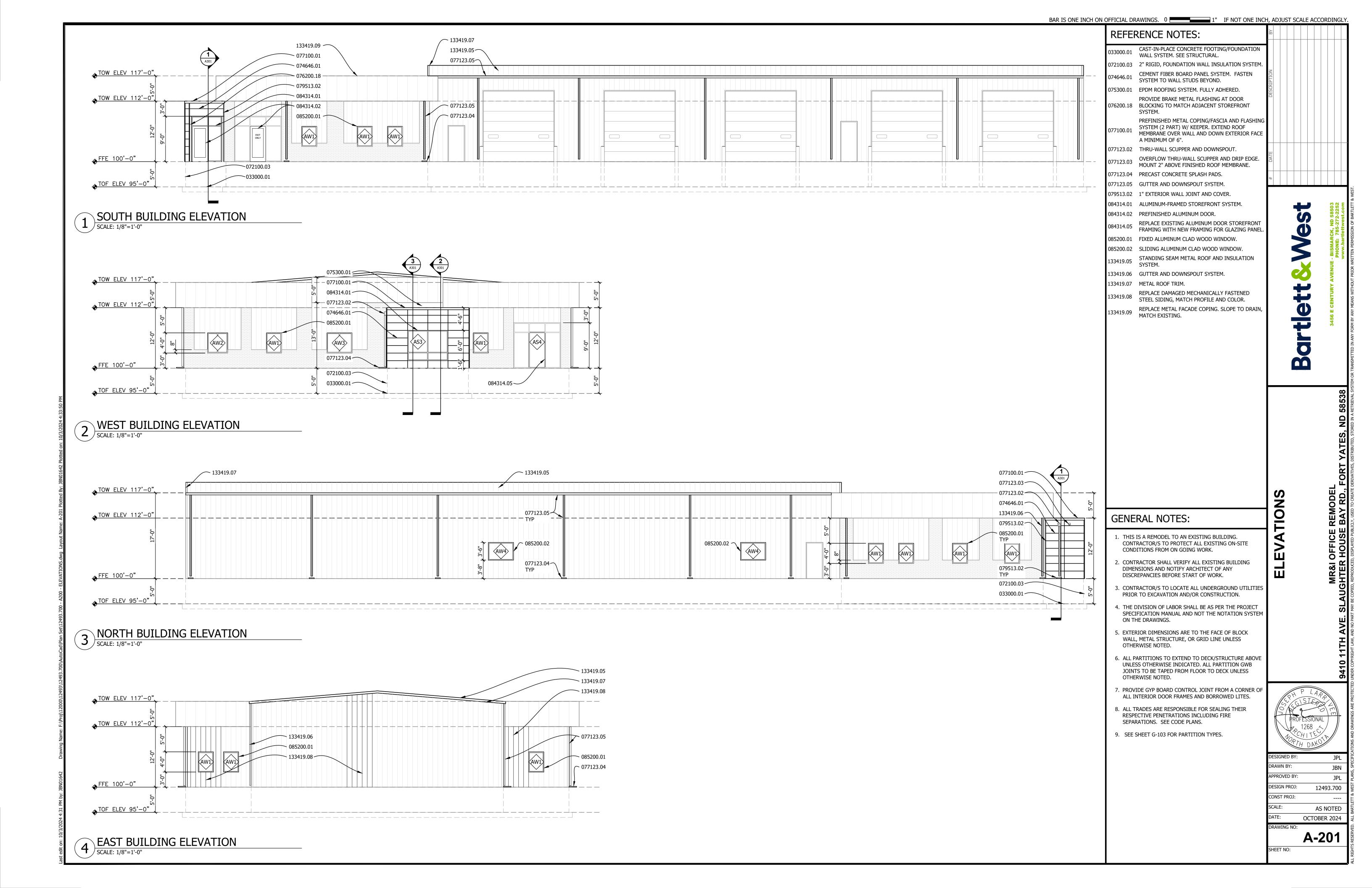
GROUND LEVEL DEMOLITION PLAN

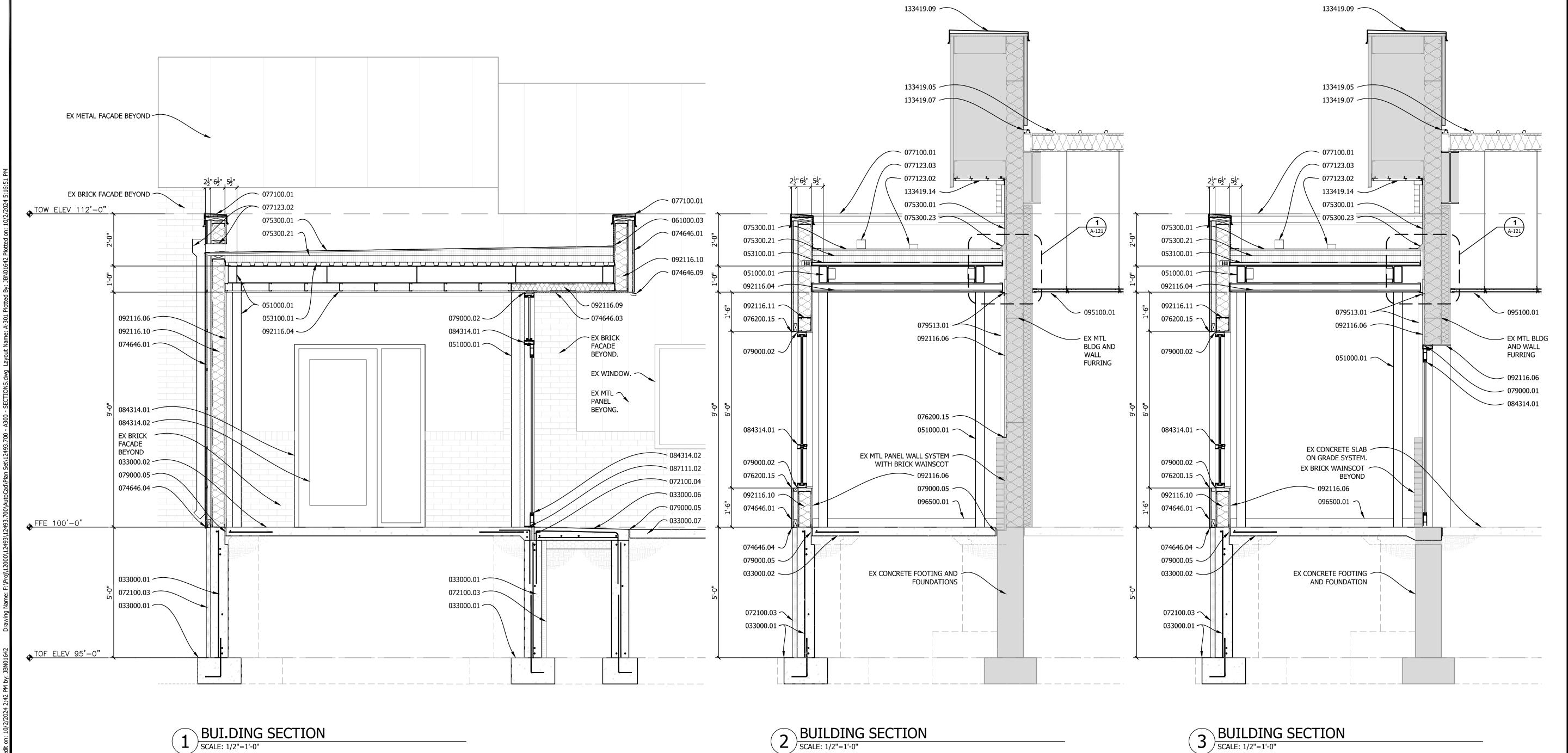
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Bartlett & West

SECTIONS

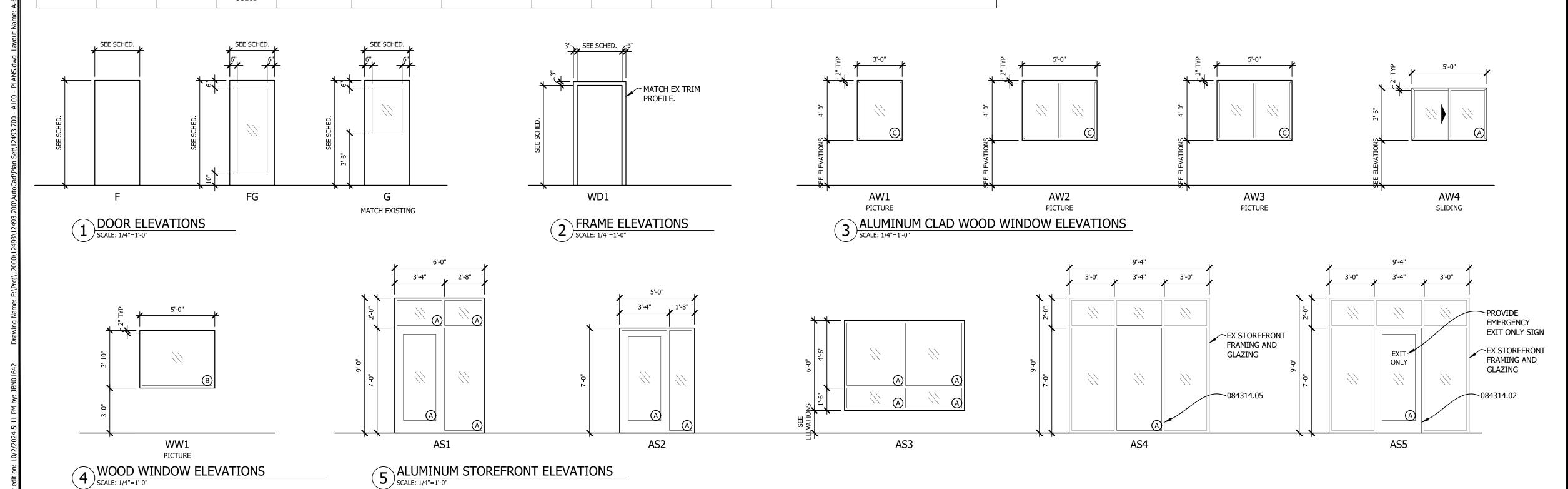
MR&I OFFICE REMODE

DESIGNED BY:	JPL
DRAWN BY:	JBN
APPROVED BY:	JPL
DESIGN PROJ:	12493.700
CONST PROJ:	
SCALE:	AS NOTED
DATE:	OCTOBER 2024

A-301
SHEET NO:

									DOOR	AND FRA	AME SCH	EDULE						
				DOOR						FRA	AME							
MARK		SIZE						SIZE					DETAIL		RATING	HW	REMARKS	
	WD	HGT	THK	EL	MATL	GLAZING	WD	HGT	THK	EL	MATL	HEAD	JAMB	SILL		SET		
101	3'-0"	7'-0"	1 3/4"	FG	ALUM	1" INSUL	6'-0"	9'-0"	4 1/2"	AS1	ALUM	1/A-301		1/A-301		1		
102	3'-0"	7'-0"	1 3/4"	FG	ALUM	1/4" TEMPERED	5'-0"	7'-0"	4 1/2"	AS2	ALUM	3/A-301		3/A-301		3		
103A	6'-0"	6'-8"	1 3/4"	FG	WD	EXIST	6'-4"	6'-10"	5 3/4"	WD1	WD						INSTALL SALVAGED DOOR, FRAME, HARDWARE, AND BLINDS.	
103B	3'-0"	7'-0"	1 3/4"	FG	ALUM	1" INSUL	9'-4"	9'-0"	4 1/2"	AS5	ALUM					2	INSTALL NEW DOOR IN EXIST FRAME	
104A	3'-0"	6'-8"	1 3/4"	G	WD	1/4" SAFETY	3'-4"	6'-10"	5 3/4"	WD1	WD					4	INSTALL SALVAGED WINDOW BLINDS	
110	3'-0"	6'-8"	1 3/4"	G	WD	1/4" SAFETY	3'-4"	6'-10"	5 3/4"	WD1	WD					4	INSTALL SALVAGED WINDOW BLINDS	

						DOOR HARDWARE	SCHEDULE				
HARDWARE		HINGES	_	LATCH	EXIT DEVICE	CLOSER	GASKET	SWEEP	THRESHOLD	STOPS	
SET	QTY	SIZE	DESIGN BASIS	DESIGN BASIS	DESIGN BASIS	DESIGN BASIS	DESIGN BASIS	DESIGN BASIS	DESIGN BASIS	DESIGN BASIS	REMARKS
1	AS REQR	4 ½" X 4 ½"	BB1191 NRP US32D		CD - 99 - NLOP - 626 20-079 626	4040XP - CUSH - AL	S88GR	964C	S205A		PROVIDE IVES 8103 10" PULL, 626.
2	AS REQR	4 ½" X 4 ½"	BB1191 NRP US32D		CD - 99 - EO - 626 20-079 626	4040XP - CUSH - AL	S88GR	964C	S205A		
3	AS REQR	4 ½" X 4 ½"	BB1279 US26D			4040XP - CUSH - AL					PROVIDE IVES 9103 10" PULL/PUSH BAR COMBO, 626.
4	AS REQR	4 ½" X 4 ½"	BB1279 US26D	ND53PD - RHO - 626						3211T-32D	PROVIDE DOOR SILENCERS.



REFERENCE NOTES:

084314.02 PREFINISHED ALUMINUM DOOR.

REPLACE EXISTING ALUMINUM DOOR STOREFRONT 084314.05 REPLACE EALSTING ALGORITHOUS SOCIETY FRAMING WITH NEW FRAMING FOR GLAZING PANEL.

GENERAL NOTES:

- 1. THIS IS A REMODEL TO AN EXISTING BUILDING. CONTRACTOR/S TO PROTECT ALL EXISTING ON-SITE CONDITIONS FROM ON GOING WORK.
- 2. CONTRACTOR SHALL VERIFY ALL EXISTING BUILDING DIMENSIONS AND NOTIFY ARCHITECT OF ANY DISCREPANCIES BEFORE START OF WORK.
- 3. CONTRACTOR/S TO LOCATE ALL UNDERGROUND UTILITIE PRIOR TO EXCAVATION AND/OR CONSTRUCTION.
- 4. THE DIVISION OF LABOR SHALL BE AS PER THE PROJECT SPECIFICATION MANUAL AND NOT THE NOTATION SYSTE ON THE DRAWINGS.
- 5. EXTERIOR DIMENSIONS ARE TO THE FACE OF BLOCK WALL, METAL STRUCTURE, OR GRID LINE UNLESS OTHERWISE NOTED.
- 6. ALL PARTITIONS TO EXTEND TO DECK/STRUCTURE ABOVE UNLESS OTHERWISE INDICATED. ALL PARTITION GWB JOINTS TO BE TAPED FROM FLOOR TO DECK UNLESS OTHERWISE NOTED.
- 7. PROVIDE GYP BOARD CONTROL JOINT FROM A CORNER O ALL INTERIOR DOOR FRAMES AND BORROWED LITES.
- 8. ALL TRADES ARE RESPONSIBLE FOR SEALING THEIR RESPECTIVE PENETRATIONS INCLUDING FIRE SEPARATIONS. SEE CODE PLANS.
- 9. SEE SHEET G-103 FOR PARTITION TYPES.

ROOM SCHEDULE KEY

FLOORING:

EX CONC - EXIST CONCRETE FLOORING CONC/S - SEALED CONCRETE CONC/H - HARDENED CONCRETE CPT - CARPET

WO CPT - WALK OFF CARPET EX PT - EXISTING PORCELAIN TILE OVER CONC. LVT - LUXURY VINYL TILE

VCT - VINYL COMPOSITION TILE

RCB - RUBBER COVE BASE ECB - EPOXY COVE BASE

EX GWB - EXISTING GYPSUM WALL BOARD PARTITION EX GWB/P - EXISTING GYPSUM WALL BOARD PARTITION,

GWB/P - GYPSUM WALL BOARD PARTITION, PAINT EX MTL PNL - EXISTING METAL LINER PANEL

CEILING:

EX MTL PNL - EXISTING METAL LINER PANEL EX GWB CLG - EXISTING GYPSUM WALL BOARD CEILING GWB CLG/P - GYPSUM WALL BOARD CEILING, PAINTED ACT - LAY IN ACOUSTICAL CEILING TILE AND GRID

GLAZING SCHEDULE KEY

- (A) IG-1 INSULATED GLASS UNITS:

MONOLITHIC SAFETY GLASS

- IG-2 INSULATED GLASS UNITS WITH
- BETWEEN GLASS BLINDS.

SUPPLIER TO REVIEW GLAZING FOR COMPLIANCE WITH APPLICABLE CODES AND STANDARDS.

RAWN BY: APPROVED BY: JPL DESIGN PROJ: 12493.700 CONST PROJ:

SCHE

AS NOTED OCTOBER 2024 A-601

SHEET NO:

- INSTALL SALVAGED WOOD DOOR, FRAME, AND 081416.02 HARDWARE. SEE DOOR AND HARDWARE
- 085200.01 FIXED ALUMINUM CLAD WOOD WINDOW.

- 123200.01 SURFACE COUNTERTOP WITH BACK AND SIDE

- SOLID SURFACE COUNTERTOP WITH BACK AND
- 123200.11 SIDE SPLASHES ON HEAVY DUTY METAL WALL
- 260000.07 MONITOR AND WALL BRACKET. SEE ELECTRICAL.

0 m

TIONS Σ 0

- CONTRACTOR/S TO PROTECT ALL EXISTING ON-SITE
- 2. CONTRACTOR SHALL VERIFY ALL EXISTING BUILDING DIMENSIONS AND NOTIFY ARCHITECT OF ANY DISCREPANCIES BEFORE START OF WORK.
- 3. CONTRACTOR/S TO LOCATE ALL UNDERGROUND UTILITIES
- 4. THE DIVISION OF LABOR SHALL BE AS PER THE PROJECT SPECIFICATION MANUAL AND NOT THE NOTATION SYSTEM
- WALL, METAL STRUCTURE, OR GRID LINE UNLESS
- UNLESS OTHERWISE INDICATED. ALL PARTITION GWB JOINTS TO BE TAPED FROM FLOOR TO DECK UNLESS
- 7. PROVIDE GYP BOARD CONTROL JOINT FROM A CORNER OF ALL INTERIOR DOOR FRAMES AND BORROWED LITES.
- 8. ALL TRADES ARE RESPONSIBLE FOR SEALING THEIR RESPECTIVE PENETRATIONS INCLUDING FIRE



DESIGNED BY:	JPL
DRAWN BY:	JBN
APPROVED BY:	JPL
DESIGN PROJ:	12493.700
CONST PROJ:	
SCALE:	AS NOTED
DATE:	OCTOBER 2024

A-701

SHEET NO:

INDICATES CANDELA RATING

P-TYPE TRAP

——— PIPE GUIDE

0

MOUNTING HEIGHT AFF TO TOP OF OF DEVICE THESE LETTERS ADJACENT TO ANY SYMBOL INDICATES TAMPER PROOF

NATHAN J GAFFREY PE-8361 DATE

SDM/NJG PPROVED BY: ESIGN PROJ: 12493.700 AS NOTED

MEP000

OCTOBER 2024

UNDERFLOOR IMPROVEMENTS. P/C TO FIELD COORDINATE ALL REQUIREMENTS AND DIRECTLY SUB-CONTRACT WITH G/C TO PROVIDE ALL CONCRETE REMOVAL REQUIRED IN AREA INDICATED.

GENERAL NOTES:

- 1. P/C TO COORDINATE WORK WITH ALL OTHER TRADES.
- 2. ALL EX UNDERFLOOR PIPING TO REMAIN UNLESS NOTED OTHERWISE.
- 3. THE EX CONDITIONS HAVE BEEN SHOWN BASED ON CASUAL ON-SITE INVESTIGATION WITH NO GUARANTEE OF THEIR ACCURACY, CONTRACTOR TO BE RESPONSIBLE TO FIELD VERIFY EXACT CONDITIONS.

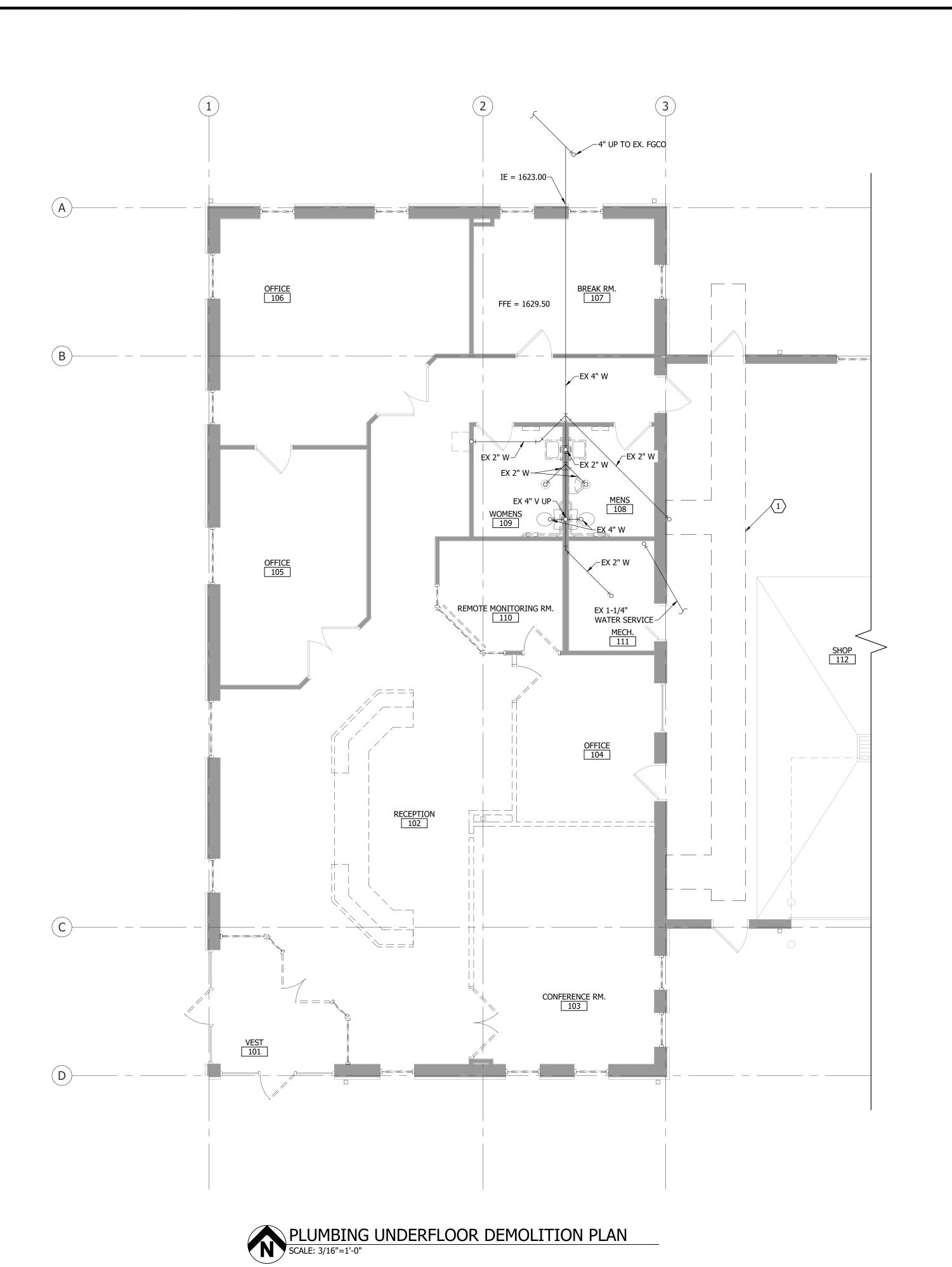
RFLO PLAN PLUMBING UNDE DEMOLITION F

SDM/NJG

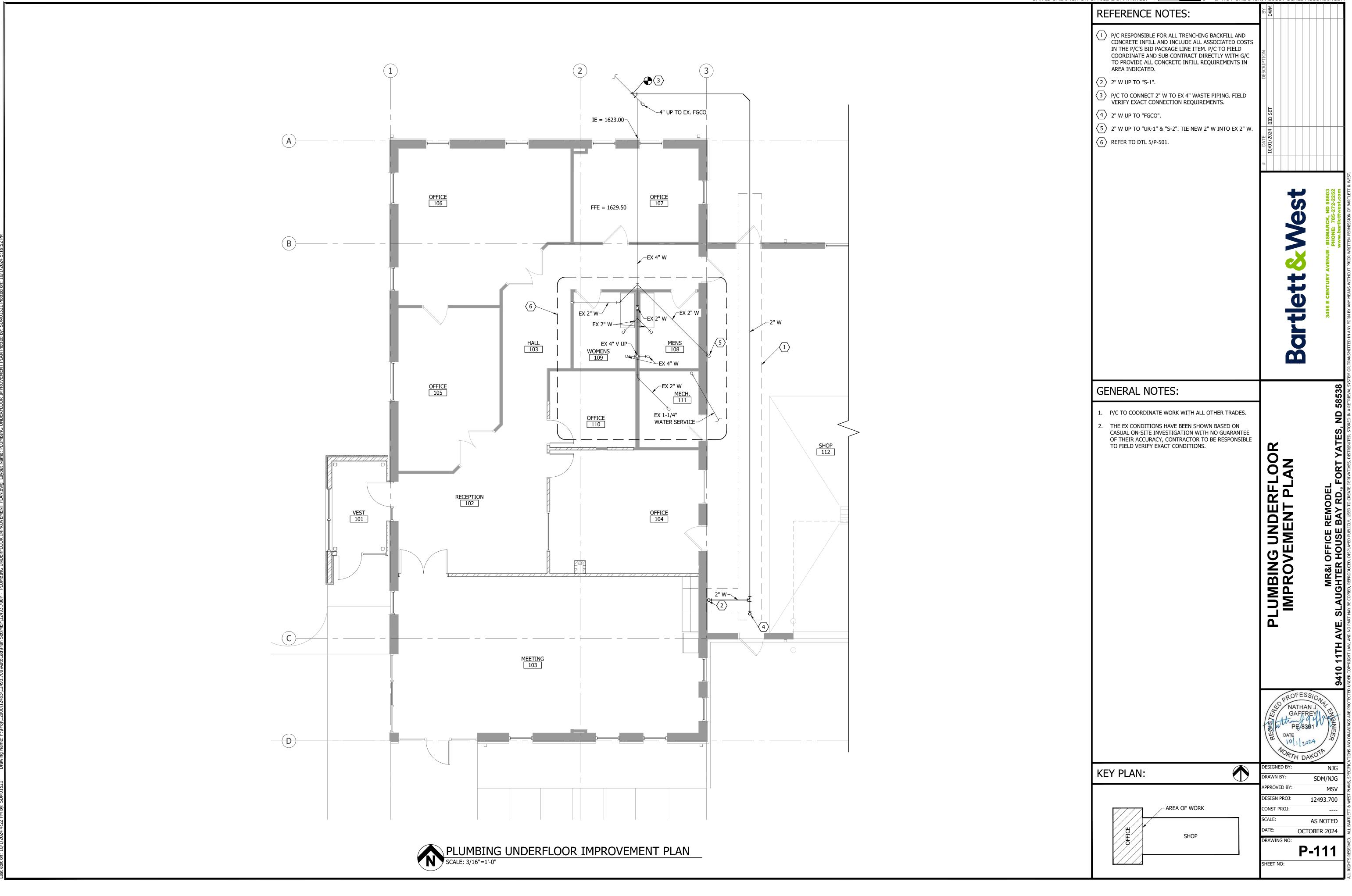
12493.700

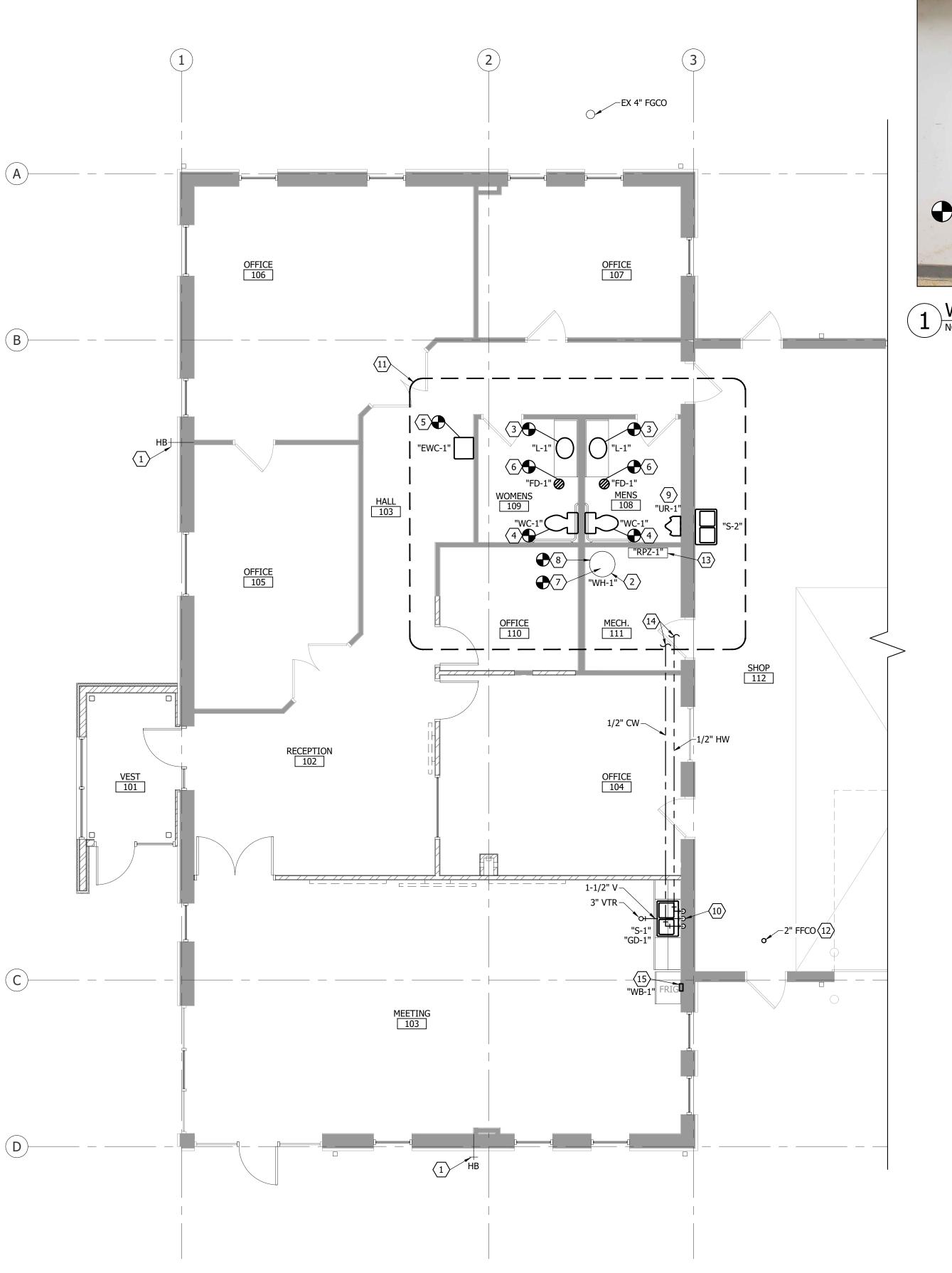
AS NOTED

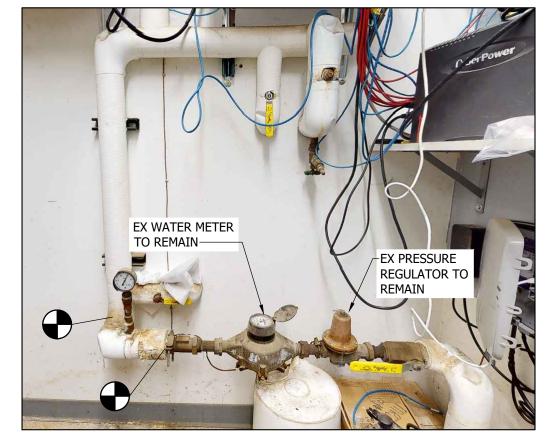
KEY PLAN: OCTOBER 2024 P-101











WATER SERVICE ENTRANCE-MODIFIED (1) NOT TO SCALE

REFERENCE NOTES:

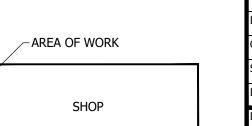
- $\langle 1 \rangle$ Ex hose bib and associated piping to remain.
- $\langle 2 \rangle$ NEW WATER HEATER "WH-1". CONNECT TO EXISTING DOMESTIC COLD WATER SUPPLY AND HOT WATER SUPPLY. REFER TO DETAIL 3/P501.
- $\overline{3}$ P/C TO CONNECT NEW 1/2" CW, 1/2" HW, 2" W, TO EX PIPING STUB-OUTS. PROVIDE NEW SHUTOFF VALVE AND WATER SUPPLY LINE.
- 4 P/C TO CONNECT NEW 1/2" CW, & 4" W TO EX PIPING STUB-OUTS. PROVIDE NEW SHUTOFF VALVE AND WATER SUPPLY LINE.
- $\overline{\langle 5 \rangle}$ P/C TO CONNECT NEW 1/2" CW, & 1-1/2" W TO EX PIPING STUB-OUTS.
- $\langle 6 \rangle$ P/C to provide New 4 inch diameter floor drain COVER OVER EX FLOOR DRAIN.
- 7 CONNECT HW TO OUTLET OF NEW WATER HEATER AND PROVIDE SHUT OFF VALVE AND UNION.
- 8 CONNECT CW TO NEW DOMESTIC COLD WATER SERVICE DOWN STREAM FROM WATER METER AND PRV.
- 9 CONNECT NEW 1-1/2" V AND 2" W SERVING "UR-1" INTO EX 1-1/2" V AND EX 2" W SERVING NEW "S-2". CONNECT NEW 3/4" CW INTO "UR-1".
- (10) CONNECT NEW 1/2" CW, 1/2" HW, 1-1/2" V DOWN TO
- REFER TO DTL 5/P-501.
- 12 REFER TO DTL 2/P-501.
- (13) P/C TO PROVIDE NEW RPZ TO EX WATER SERVICE PIPING REFER TO DTL 1/P-501.
- (14) P/C TO CONNECT 1/2" CW, HW PIPING TO EX PIPING IN MECH RM 111. FIELD VERIFY EXACT CONNECTION REQUIREMENTS.
- 15 PROVIDE "WB-1" OATEY I2K ICE MAKER BOX OR PREAPPROVED EQUAL. MOUNT AT 2 FEET AFF BEHIND REFRIGERATOR. REFER TO DTL 5/501.

GENERAL NOTES:

- 1. P/C TO COORDINATE WORK WITH ALL OTHER TRADES.
- 2. THE EX CONDITIONS HAVE BEEN SHOWN BASED ON CASUAL ON-SITE INVESTIGATION WITH NO GUARANTEE OF THEIR ACCURACY, CONTRACTOR TO BE RESPONSIBLE TO FIELD VERIFY EXACT CONDITIONS.

MENT PLUMBING IMPROVE

KEY PLAN:



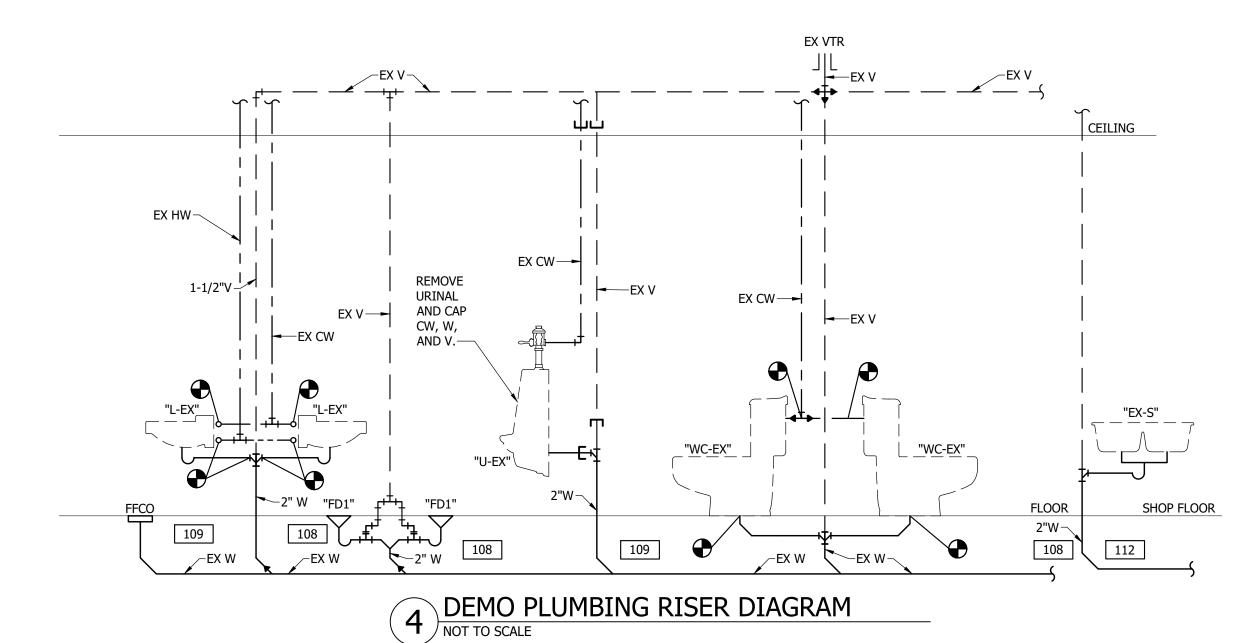
12493.700 AS NOTED

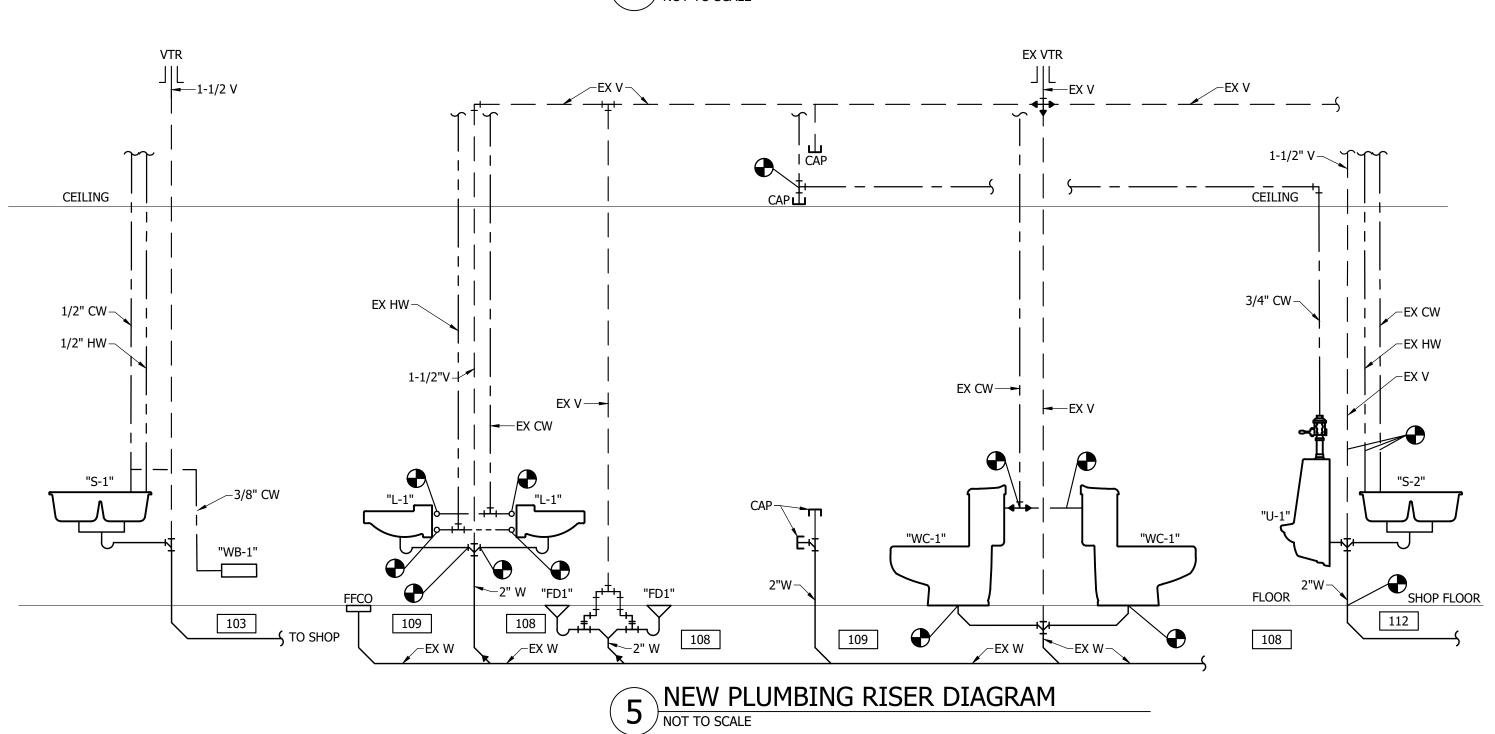
OCTOBER 2024

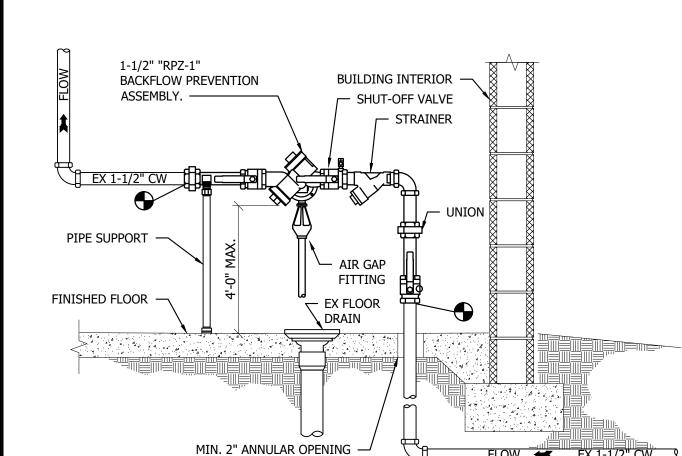
P-112

PLUMBING IMPROVEMENT PLAN
SCALE: 3/16"=1'-0"

PLUM	BING FIXTUR	E SCHEDU	LE							
MARK	FIXTURE	MFGR	MODEL	MOUNTING	ROU	GH-IN SI	ZES (INC	HES)	DESCRIPTION	ACCESSORIES
					CW		WASTE			
"WC-1"	WATER CLOSET (ADA)	AMERICAN STANDARD	270AB.101 CADET	FLOOR	1/2	-	4	2	VITREOUS CHINA, FLOOR OUTLET, 1.28 GPF.	SEAT: AMERICAN STANDARD 5321.110
"UR-1"	URINAL (ADA)	AMERICAN STANDARD	6590.001 WASHBROOK	WALL	3/4	-	2	2	FLUSH VALVE, ELONGATED RIM, VITREOUS CHINA, 0.75 INCH TOP SPUD, 0.125 GPF WASHOUT FLUSH ACTION	FLUSH VALVE: AMERICAN STANDARD #605.013.002 (TOP SPUD) MANUAL MOUNT AT ADA HEIGHT
"L-1"	LAVATORY (ADA)	AMERICAN STANDARD	427.444EC CADET	COUNTERTOP	1/2	1/2	2		SELF-RIMMING, STAINLESS STEEL, 4-INCH CENTER FAUCET HOLES, FRONT OVERFLOW AND CHROME PLATED DRAIN TAIL PIECE	WASTE & SUPPLY PIPING COVER: TRUEBRO MODEL #102 E-Z FAUCET: AMERICAN STANDARD 2385.003 RELIANT+ (GRID DRAIN) INSULATION KIT: TRUEBRO LAV-GUARD 2 E-Z
"S-1"	SINK (ADA)	ELKAY	LRADQ331950	COUNTERTOP	1/2	1/2	2		DUAL COMPARTMENT, SELF-RIMMING, 5-INCH BOWL DEPTH, TYPE 304(18-8) STANLESS STEEL SINK WITH HOLES IN DECK TO MATCH FAUCET	WASTE & SUPPLY PIPING COVER: TRUEBRO MODEL #102 E-Z DRAIN: ELKAY LK-35 FAUCET: AMERICAN STANDARD LKD2423BHC (LESS HANDSPRAY) BOTTLE FILLER: BRITA USS-323 2 STAGE FILTRATION SYSTEM FOOD DISPOSAL: INSINKERATOR BADGER 5 WITH POWER CORD
"S-2"	SINK	MUSTEE	27F	FLOOR	1/2	1/2	2		DUAL COMPARTMENT, FREE-STANDING, ONE-PIECE MOLDED CONSTRUCTION, TWIN DRAIN WASTE COUPLING, HEAVY GAUGE STEEL LEGS WITH LEVELING BUILD IN, P-TRAP, MOLD AND MILDEW RESISTANT	WASTE & SUPPLY PIPING COVER: TRUEBRO MODEL #102 E-Z FAUCET: MUSTEE MODEL 93.600
"EWC-1"	ELECTRIC WATER COOLER	HALSEY TAYLOR	HTHB-HAC8SS-W	WALL	3/4	-	1-1/2	1-1/2	ELECTRIC WATER COOLER, BUBBLER MECHANICAL PUSHBAR ACTIVATION,	REFRIGERANT: R-134A
"WH-1"	ELECTRIC WATER HEATER	RELIANCE	6 50 EORS	FLOOR	3/4	-	-	-	ELECTRIC WATER HEATER, 240 V/ 1 PHASE, 4500 W DUAL HEATING ELEMENT, GLASS LINED TANK, 50 GALLON CAPACITY.	PROVIDE WITH FACTORY TEMPERATURE AND PRESSURE RELIEF VALVE. PROVIDE THERM-X-TROL #ST-5-DD EXPANSION TANK 2.0 GALLON VOLUME.
"RPZ-1"	REDUCED PRESSURE BACKFLOW PREVENTER	WATTS	919QT-S	WALL	1	-	-	-	REDUCED PRESSURE BACKFLOW PREVENTER WITH STRAINER AND SHUT OFF VALVES.	-

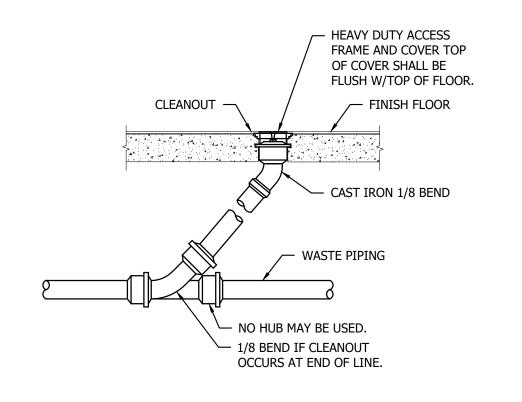






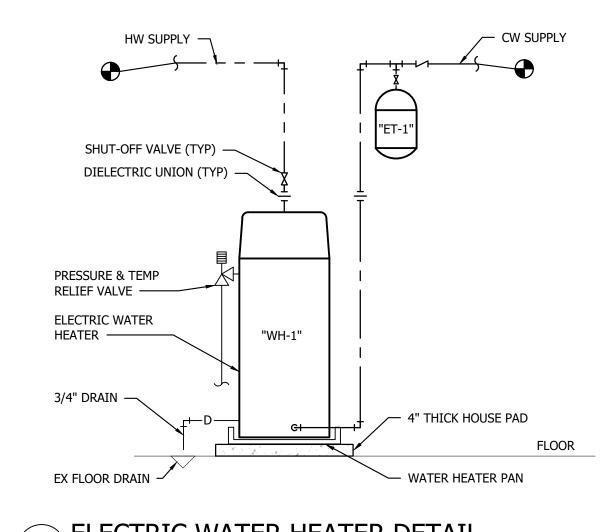
RPZ ASSEMBLY WITH STRAINER DETAIL

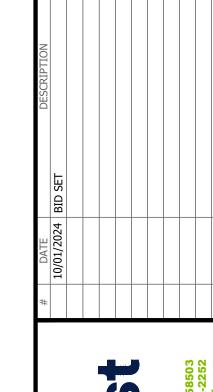
NOT TO SCALE



PINISHED FLOOR CLEANOUT DETAIL

NOT TO SCALE





Bartlett & West 3456 E CENTURY AVENUE - BISMARCK, ND 5850

PLUMBING DETAILS SCHEDULES & RISERS

PE-8361

DATE

NORTH DAKOTA

DESIGNED BY: NJG

DRAWN BY: SDM/NJG

APPROVED BY: MSV

DESIGN PROJ: 12493.700

CONST PROJ: ---
SCALE: AS NOTED

DATE: OCTOBER 2024

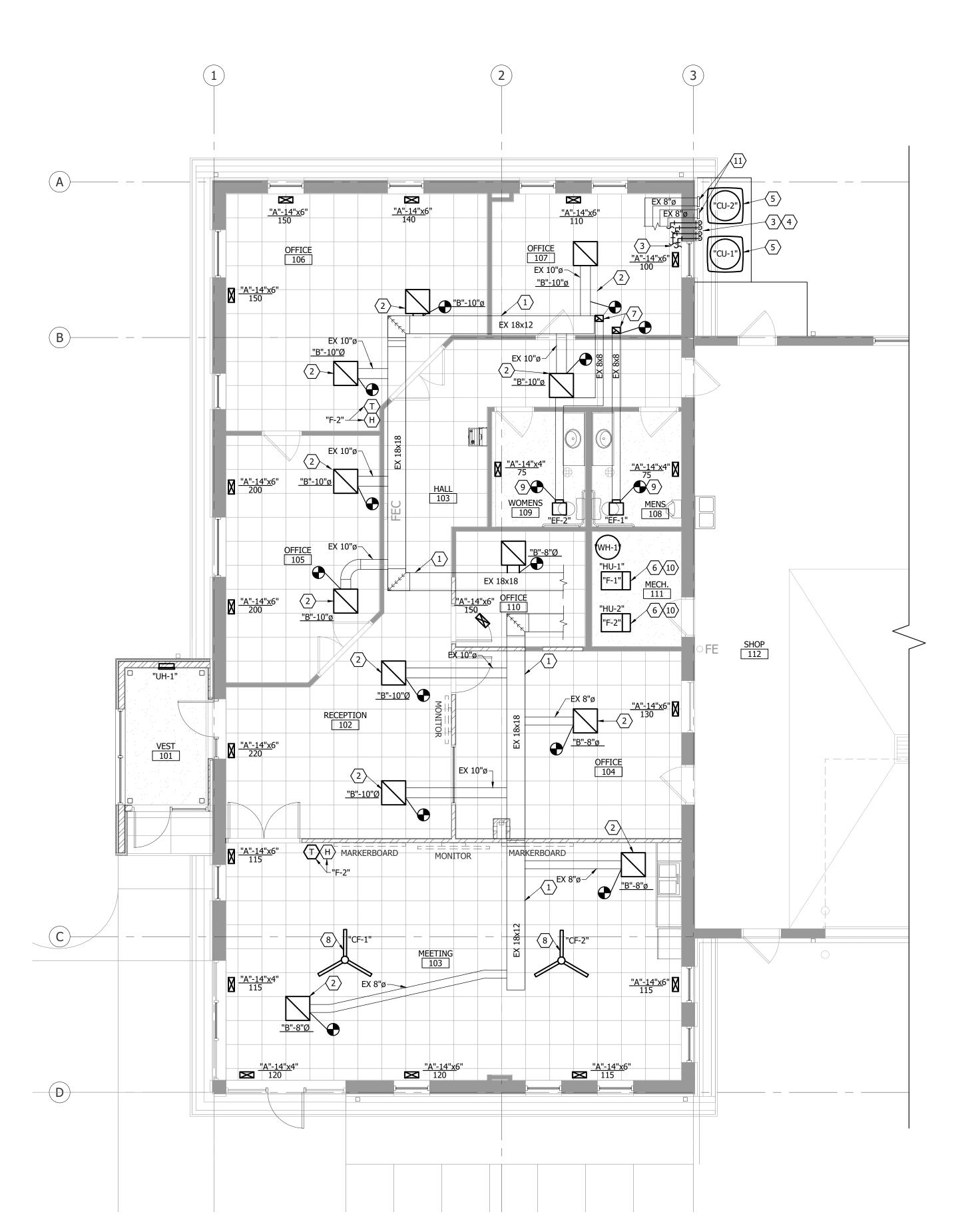
P-501
SHEET NO:

3 ELECTRIC WATER HEATER DETAIL

NOT TO SCALE



M-112



MECHANICAL IMPROVEMENT PLAN

SCALE: 3/16"=1'-0"

NOTES:

- 1. PROVIDE WITH STAINLESS-STEEL HEAT EXCHANGER.
- 2. TWO-STAGE PROPANE GAS VALVE.
- 3. INSULATED HEAT EXCHANGER AND BLOWER SECTION. 4. PROPANE GAS PRESSURE REGULATOR, FIELD VERIFY PROPANE GAS INLET PRESSURE.
- 5. THERMOSTAT-WALL-MOUNTED, 7-DAY PROGRAMABLE.
- R-32 REFRIGERANT.
- 7. SINGLE STAGE SCROLL COMPRESSOR.
- 8. PROVIDE APRILAIRE MODEL 700 HUMIDIFIERS WITH REMOTE DIGITAL DISPLAY HUMIDIFIER CONTROL OR PREAPPROVED EQUAL. 9. PROVIDE REFRIGERANT LEAK DETECTION SENSORS IN SUPPLY DUCT WORK OF EACH FURNACE UNIT. REFRIGERANT LEAK DETECTION SHALL BE IN COMPLIANCE WITH ASHRAE STANDARD 15.
- 10. PROVIDE VISUAL AND AUDIBLE ALARM FOR REFRIGERANT LEAK DETECTION.
- 11. PROVIDE F-1 AND F-2 WITH MERV-8 AIR FILTERS.

FAN S	CHEDULE									
MARK	MFGR	MODEL	AIRFLOW	E.S.P.	MOTOR				NOTES	
MAKK	MEGR	MODEL	(CFM)	(IN. WC.)	HP	FLA	VOLTS	PHASE		
EF-1	GREENHECK	SP-A200	140	0.30	0.01	0.46	120	1	1,2	
EF-2	GREENHECK	SP-B110ES	70	0.30	0.02	0.27	120	1	1,2	
CF-1	MONTE CARLO	5HV52RZW	4400		FRAC		120	1	3,4,5	
CF-2	MONTE CARLO	5HV52RZW	4400		FRAC		120	1	3,4	

- INTEGRAL DISCONNECT SWITCH.
- GRAVITY BACKDRAFT DAMPER.
- 3. PROVIDE WITH A SINGLE LEVITON DSF01-10Z SOLID STATE SPEED CONTROL OR PREAPPPROVED EQUAL TO
- CONTROL BOTH "CF-1" AND "CF-2" SIMULTANOUSLY.

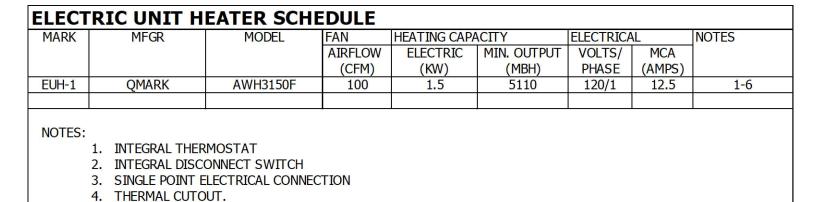
TF - LAY-IN FLOOR MOUNT

E - EXPOSED DUCT MOUNT

S - SURFACE MOUNT

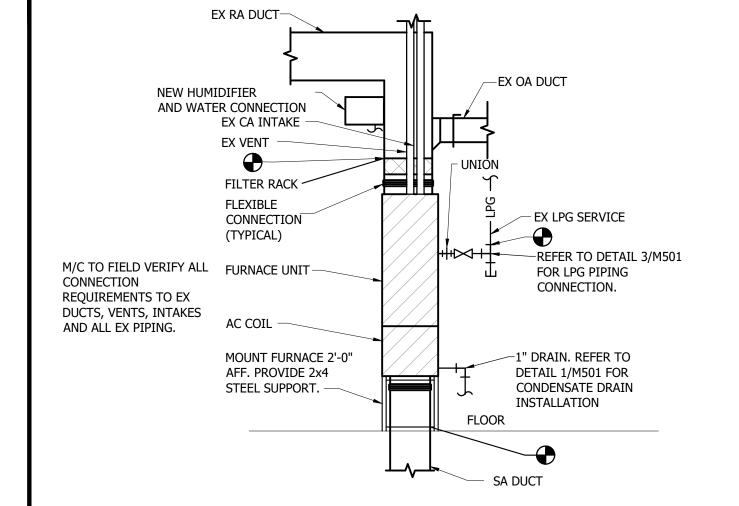
- 4. PROVIDE WITH 4-1/2 INCH DOWNROD AND T-BAR CEILING MOUNTING HARDWARE.
- 5. FAN AND SOLID STATE SPEED CONTROL TO BE WHITE IN COLOR.

GRILL	ES, REGISTE	RS, & DIF	FUSERS	SCHEDULE			
MARK	MFGR	MODEL	SERVICE	FACE SIZE	FRAME TYPE	FINISH	VOLUME CONTROL
Α	TITUS	CT-PP-0	SA	REFER TO PLANS	TF	WHITE	YES
В	TITUS	50F	RA	24x24	T, S	WHITE	YES
ABBREVIA	TIONS: T - LAY-IN CEILING M	IOUNT					



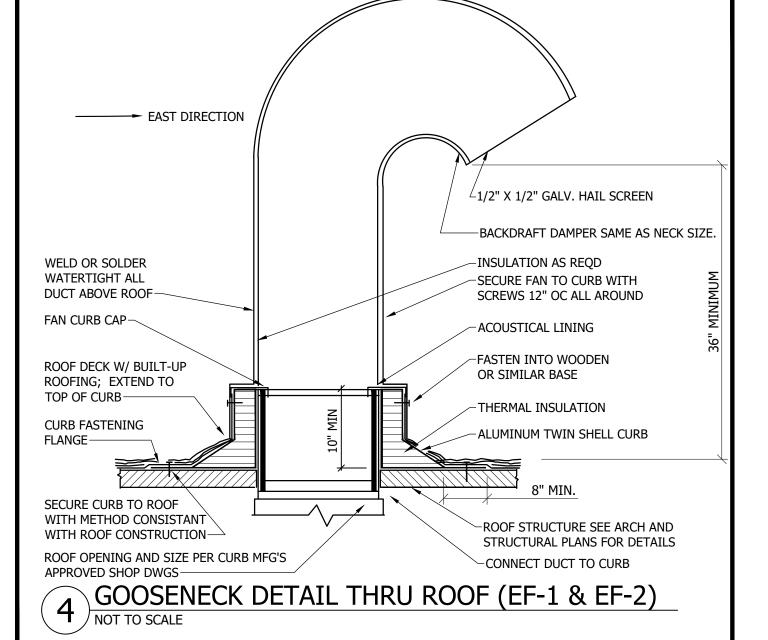
5. WALL RECESSED MOUNTED.

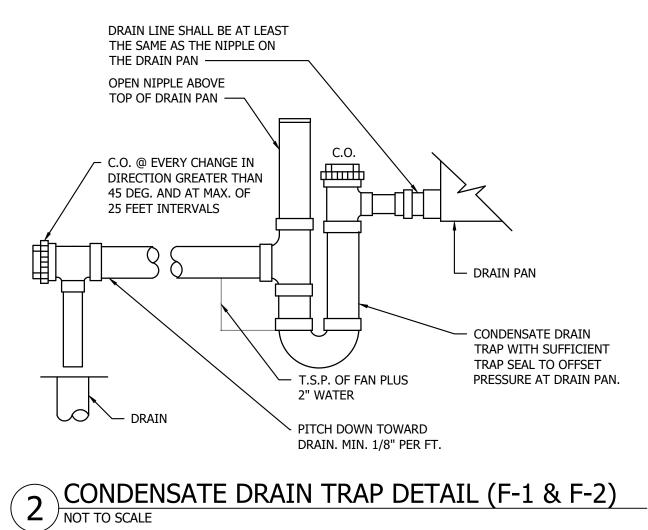
6. COLOR SHALL BE WHITE.

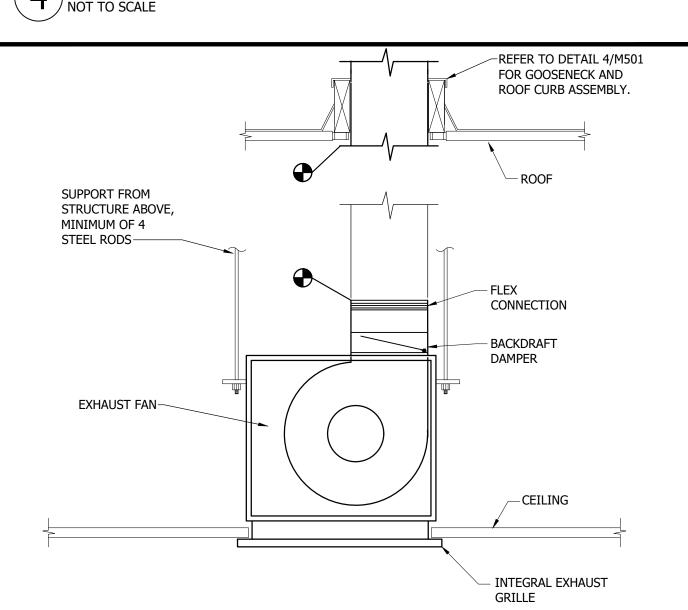


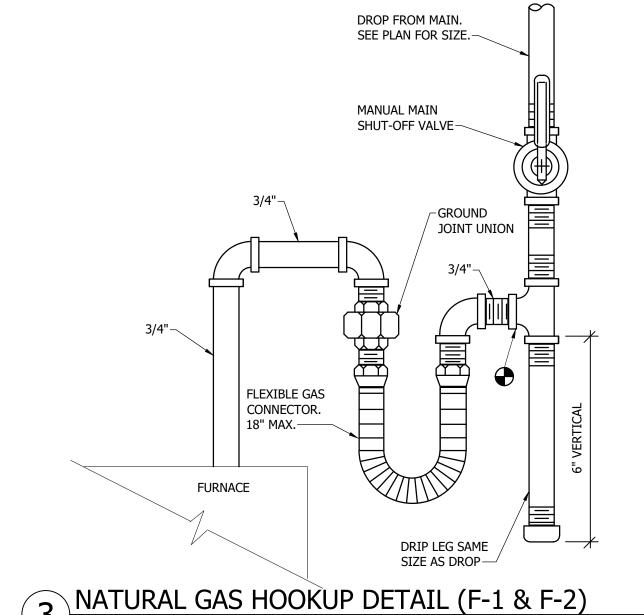
BAR IS ONE INCH ON OFFICIAL DRAWINGS. 0 _______1" IF NOT ONE INCH, ADJUST SCALE ACCORDINGLY.

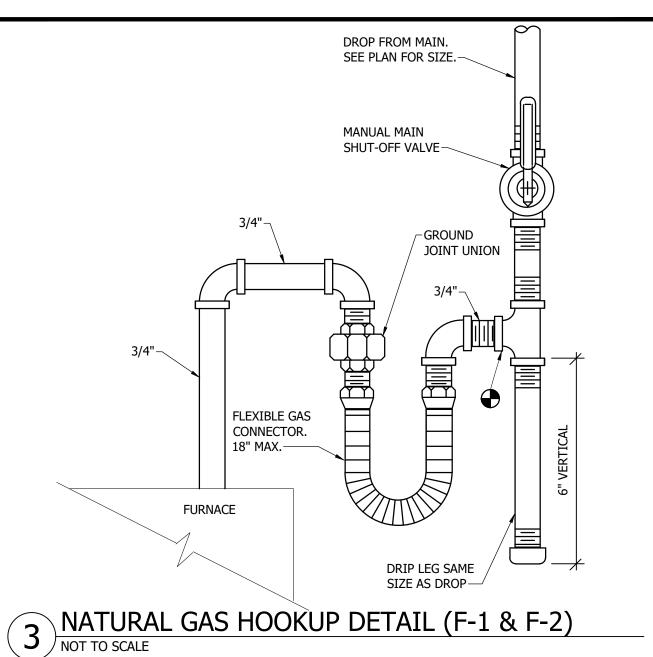
DOWN FLOW FURNACE DETAIL (F-1 & F-2)
NOT TO SCALE











RAWN BY: SDM/NJG APPROVED BY: MSV ESIGN PROJ: 12493.700 CONST PROJ: AS NOTED OCTOBER 2024

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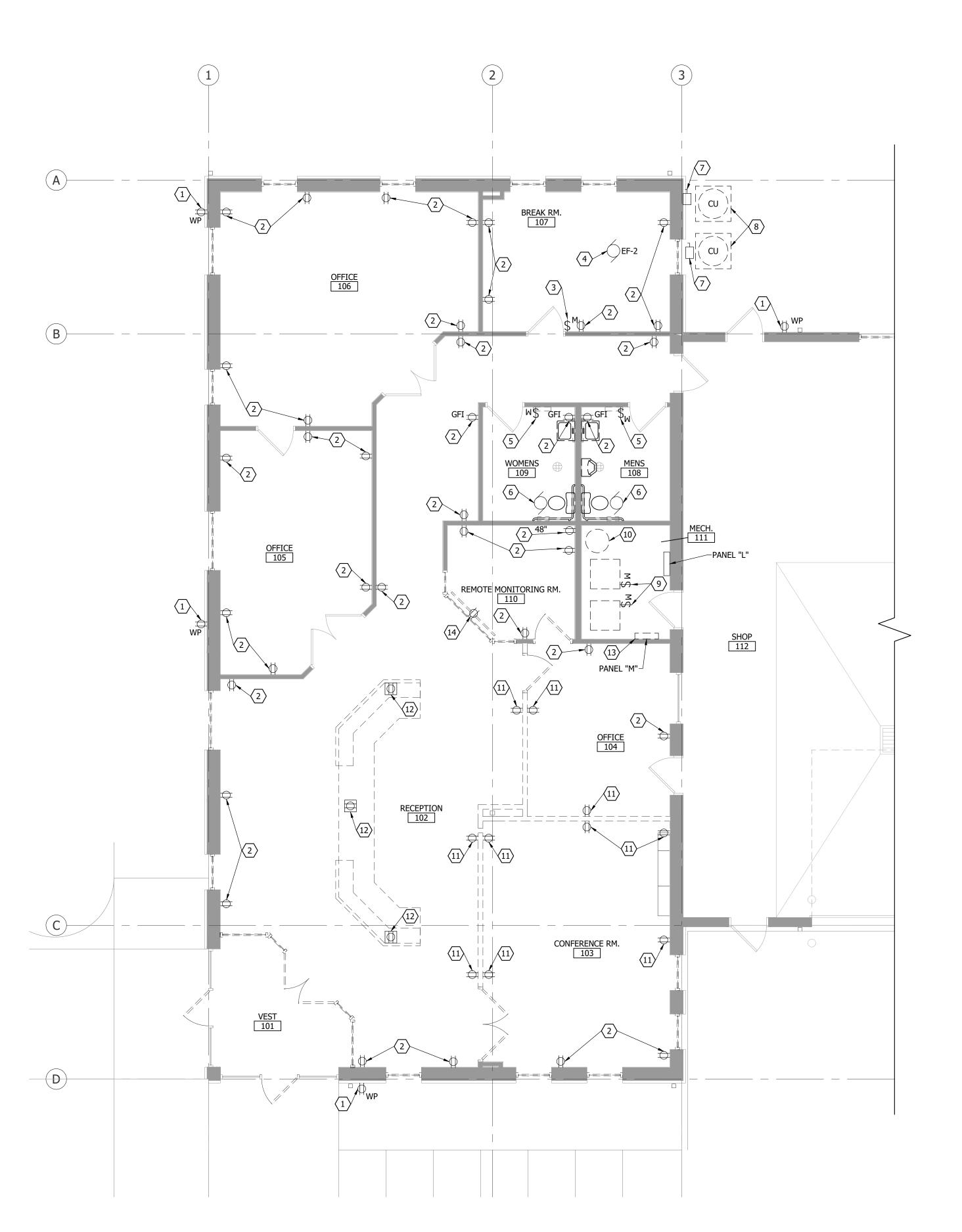
SHANICAL DE SCHEDUL

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ME

M-501 SHEET NO:

5 EXHAUST FAN DETAIL (EF-1 & EF-2)
NOT TO SCALE



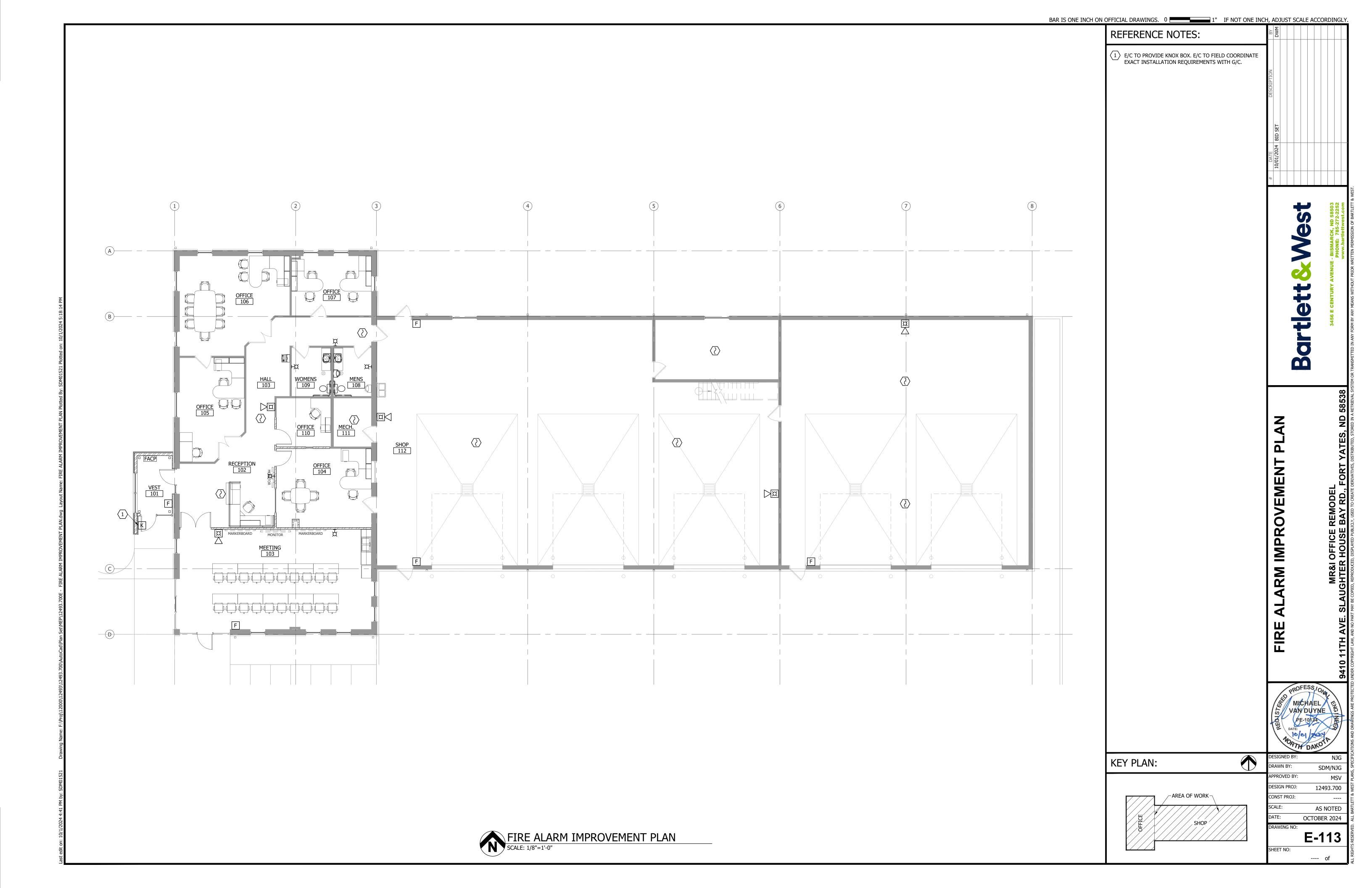
E-102

---- of

POWER DEMOLITION PLAN

SCALE: 3/16"=1'-0"

---- of



DESIGN PROJ:

CONST PROJ:

SHEET NO:

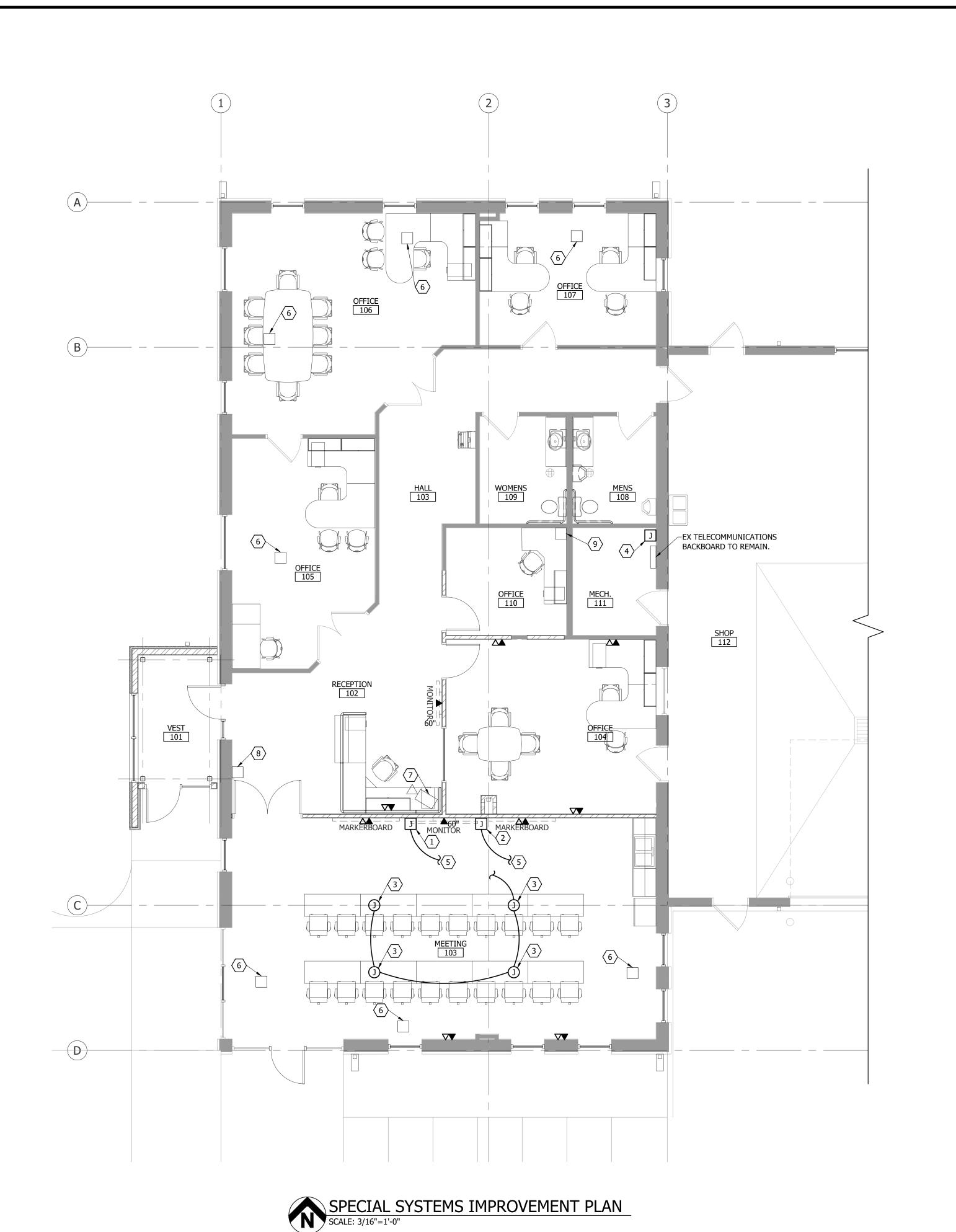
AREA OF WORK

12493.700

AS NOTED
OCTOBER 2024

E-114

---- of



PEAK DEMAND (FROM	
UTILITY)	96 A
ADDITIONAL LOAD	83 A
TOTAL	179 A

LIGHTING	CONTROL DEVI	CE SCHEDULE		
MARK	MANUFACTURER	CATALOG NUMBER	DISCRIPTION	NOTES
\$va	LEGRAND	CS-50-W	PIR WALL SWITCH VACANCY SENSOR	1
\$ _{LV}	NLIGHT	nPODM GFX	WALL MOUNT 3.5" FULL-COLOR TOUCHSCREEN CONTROLLER; 16 SCENE	1
\$ _{oc}	LEGRAND	PW-100-W	PASSIVE INFRARED WALL SWITCH OCCUPANCY SENSOR	1
© _{"OC1"}	LEGRAND	DT-355	CEILING MOUNT DUAL TECHNOLOGY LINE VOLTAGE CEILING SENSOR	1
© "OC2A"	LEGRAND	DT-355	CEILING MOUNT DUAL TECHNOLOGY LINE VOLTAGE CEILING SENSOR	1
OC2B"	LEGRAND	DT-355	CEILING MOUNT DUAL TECHNOLOGY LINE VOLTAGE CEILING SENSOR	1
© _{"OC2C"}	LEGRAND	DT-355	CEILING MOUNT DUAL TECHNOLOGY LINE VOLTAGE CEILING SENSOR	1
© _{"OC2D"}	LEGRAND	DT-355	CEILING MOUNT DUAL TECHNOLOGY LINE VOLTAGE CEILING SENSOR	1
PP	NLIGHT	nPP16 D	POWER/RELAY PACK; 0-10V DIMMING OUTPUT	1
NOTES: 1. EQ	UIVALENT MANUFACTUR	ERES: ENCELIUM BY OSRAM	1; NX BY HUBBELL; WATTSTOPPER BY LEGRAND	

	SERVED CONDENSING UNIT	VOLTAGE	AMP	FUSE				NOTES
	CONDENSING LINIT		AMP	POLE	AMP	TYPE	NEMA TYPE	
ייחכ איי כ	COMPENSING ONLY	240	30	2	-	-		
D3-2 C	CONDENSING UNIT	240	30	2	-	_		
"DS-3"	WATER HEATER	240	60	1	u u	Т		
NOTES:								

NEW PANEL "L" L	OADS			
LOAD	CONNECTED	NEC DEMAND	DEMAND	DEMAND
CLASSIFICATION	LOAD	FACTOR	LOAD	AMPS
F-1	1176 VA	100%	1176 VA	5 A
F-2	1176 VA	100%	1176 VA	5 A
WH-1	4500 VA	100%	4500 VA	40 A
CU-1	2364 VA	125%	2364 VA	12.5 A
CU-2	2364 VA	100%	2364 VA	10 A
TOTAL:	11,580 VA		11,580 VA	72.5 A

NEW PANEL "M" LOADS	CONNEC	NEC		
	TED	DEMAND	DEMAND	DEMAND
LOAD CLASSIFICATION	LOAD	FACTOR	LOAD	AMPS
EF-1	250 VA	100%	250 VA	1 A
EF-2	250 VA	100%	250 VA	1 A
EUH-1	1500 VA	100%	1500 VA	7 A
OFFICE 104 RECEPTACLE	540 VA	100%	540 VA	3 A
MEETING W 103 RECEPTACLE	540 VA	100%	540 VA	3 A
RECEPTION W 102 RECEPTACLE	800 VA	100%	800 VA	4 A
MEETING E 103 RECEPTACLE	540 VA	100%	540 VA	3 A
RECEPTION E 102 RECEPTACLE	800 VA	100%	800 VA	4 A
REFRIGERATOR RECEPTACLE	700 VA	100%	700 VA	3 A
ROOM 110, 111, 104, 103 LIGHTS	800 VA	100%	800 VA	4 A
MICROWAVE RECEPTACLE	700 VA	100%	700 VA	3 A
VESTIBULE RECEPTACLE	180 VA	100%	180 VA	1 A
COUNTER/DISPOSAL RECEPTACLI	1400 VA	100%	1400 VA	6 A
TOTAL	9000 VA		9000 VA	43 A

MARK:	EX "L"						MOUNTING:		SURFACE		10K AIC RATING (MIN	[MUM)
VOLTAGE:	120/240	PHASE:	1	WIRE:	3	POLES: 42	MAIN BUSS:		400	AMP	MAIN C/B:	250 AMP
CIRC.	LOAD DESCRIP	TION		CIRC.	LOAD	PHASE LO	DAD IN VA	LOAD	CIRC.		LOAD DESCRIPTION	CIRC
NO.				BRKR	(VA)	Α	В	(VA)	BRKR.			NO
1	EXIT WALL LIG	GHTS		20A1P		0			20A1P		OFFICE LIGHTS	2
3	ENTRY & RECEPT.	LIGHTS		20A1P			0		20A1P		OFFICE & HALL LIGHTS	4
5	RECEPT. LIG	HTS		20A1P		0			20A1P	RI	EST., MECH., & MON. LIGHTS	6
7	OFFICE & CONF.	LIGHTS		20A1P			0		20A1P		VALVE & METER LIGHTS	8
9	W. EXIT OFFICE F	RECEPT.		20A1P		0			20A1P		EXIT & EMERG. LIGHTS	10
11	N.W. EXIT OFFICE	RECEPT.		20A1P			0		20A1P	S. EX	T. OFFICE, CON. & REC RECEP	Г. 12
13	RECP. & OFFICE F	RECEPT.		20A1P		0			20A1P		CONF. RECEPT.	14
15	OFFICE RECE	PT.		20A1P			0		20A1P		OFFICE RECEPT.	16
17	RECEPTION RE	CEPT.		20A1P		0			20A1P		OFFFICE RECEPT. EF-2	18
19	REMOTE MON. F	RECEPT		20A1P			0		20A1P	RES	T. RECEPT, EF-1, EWC RECEPT	. 20
21	SHOP RECEI	PT.		20A1P		0			20A1P		SHOP RECEPT.	22
23	SHOP RECEI	PT.		20A1P			0		20A1P		SHOP RECEPT.	24
25	** F -1			20A1P	1176	1176			20A1P		PF-1A & 1B	26
27	**F-2			20A1P	1176		1176		20A1P		TELEPHONE RECEPT.	28
29	WH-1			40A2P	4500	4500			20A2P		POLE YARD LIGHTS	30
31	=			-	4500		4500		-		=	32
33	**CU-1			30A2P	2364	4728		2364	30A2P		**CU-2	34
35	-			-	2364		4728	2364	-		-	36
37	SHOP LIGHT	S		20A2P		0			20A2P		SHOP LIGHTS	38
39	-			-			0		=		-	40
41	RH-1A & 1	В		20A1P		0			20A1P		SCADA - SECURITY	42
OTALS:					-	10404	10404					

EMS IN BOLD: E/C TO PROVIDE NEW CIRCUIT BREAKER FOR NEW EQUIPMENT	
ITEMS IN BOLD: E/C TO PROVIDE NEW LABEL FOR NEW EOUIPMENT.	

-			_
*ITEMS IN BOLD: E/	C TO PROVIDE NEW	LA BEL FOR NEW	EQUIPMENT.

MARK:	"M"					MOUNTING:		SURFAC	E	12K AIC RATING (MIN	IMUM))
VOLTAGE	: 120/240	PHASE: 1	WIRE:	3	POLES: 42	MAIN BUSS:		400	AMP	MAIN C/B:	250 AI	MP
CIRC.	LOAD DESCRIP	TION	CIRC.	LOAD	PHASE LO	DAD IN VA	LOAD	CIRC.		LOAD DESCRIPTION	С	CIRC
NO.			BRKR	(VA)	Α	В	(VA)	BRKR.			ľ	NO
1			20A1P		0			20A1P	VAL	.VE METER 113 & 213 RECEPT.		2
3			20A1P			0		20A1P	VAL	VE METER 113 & 213 RECEPT.		4
5	STORAGE 114 R	ECEPT.	20A1P		0			20A1P		D0-ID & 1E		6
7	STORAGE 114 R	ECEPT.	20A1P			0		20A1P		D0-1A-1B & 1C		8
9	STORAGE 114 l	_IGHTS	20A2P		0			20A2P		STORAGE 114 LIGHTS		10
11	STORAGE 114 l	_IGHTS	ĭ			0		-		STORAGE 114 LIGHTS		12
13	EUH-1		20A2P		0			50A2P		WELDER RECEPT.		14
15	=1		-			0		-		WELDER RECEPT.		10
17	AIR COMPRES	SOR	50A2P		0			50A2P		WELDER RECEPT.		18
19	-		-			0		-		WELDER RECEPT.		20
21	** EF-1		20A1P	250	250			20A1P		OUTSIDE GFCI RECEPT.		2
23	** EF -2		20A1P	250		250		20A1P		OUTSIDE GFCI RECEPT.		2
25	MD-1		20A1P		0			20A1P		SPARE		2
27	"CF-1" & "CF	-2"	20A1P	250		250		20A1P		SPARE		2
29	SPARE		20A1P		0			20A1P		SPARE		3
31	**"EUH-1	."	20A1P	1500		2040	540	20A1P		**OFFICE 104 RECEPT.		32
33	**MEETING W 103	B RECEPT.	20A1P	540	1340		800	20A1P	k*	RECEPTION W 102 RECEPT.		34
35	**MEETING E 103	RECEPT.	20A1P	540		1340	800	20A1P	*	*RECEPTION E 102 RECEPT.		36
37	**REFRIGERATOR	R RECEPT.	20A1P	700	1500		800	20A1P	**R0	OM 110, 111, 104, 103 LIGH	TS	38
39	**MICROWAVE	RECEPT.	20A1P	1000		1180	180	20A1P		**VESTIBULE RECEPT.		40
41	**COUNTER/DISPOS	SAL RECEPT.	20A1P	1400	2400		1000	20A1P		**FACP		42
OTALS:					5490	5060						

TEMS IN BOLD: E/C TO PROVID	DE NEW CIRCUIT BREAKER FOR	R NEW EQUIPMENT.
*ITEMS IN BOLD: E/C TO PRO	VIDE NEW LABEL FOR NEW EQ	UIPMENT.

THE STREET, C. TO I KOVIDE IEW EADER TOX IEW EQUIT FIEM.
E/C TO PROVIDE NEW CIRCUIT BREAKERS FOR ALL EX CIRCUITS TO BE RECONNECTED.
E/C TO PROVIDE NEW CIRCUIT BREAKERS FOR CIRCUITS LABELED AS SPARE.

LOGTITECH EQUIPMENT							
EQUIPMENT	PART NUMBER	POD MICROPHONES ADD 10 METERS OF ADDITIONAL REACH TO THE CABLING OF RALLY MIC POD OR RALLY MIC POD HUB FLOAT RALLY BAR MINI OR RALLY BAR ABOVE OR BELOW A TV OR MONITOR FOR A SLEEK INSTALLATION AND MINIMAL FOOTPRINT ALL-IN-ONE VIDEO BAR FOR MEDIUM TO LARGE ROOMS TURN ANY SPACE INTO AN APPLIANCE-BASED VIDEO CONFERENCING ROOM WITH					
NAIC DOD DENIDANIT NAOLINIT	052 000122	CEILING PENDANT MOUNT FOR RALLY MIC					
MIC POD PENDANT MOUNT	952-000123	POD MICROPHONES					
		ADD 10 METERS OF ADDITIONAL REACH TO					
RALLY MIC POD EXTENTION CABLE	952-000047	THE CABLING OF RALLY MIC POD OR RALLY					
		MIC POD HUB					
		THE CABLING OF RALLY MIC POD OR RALL MIC POD HUB FLOAT RALLY BAR MINI OR RALLY BAR ABOVE OR BELOW A TV OR MONITOR FOR A SLEEK INSTALLATION AND MINIMAL FOOTPRINT ALL-IN-ONE VIDEO BAR FOR MEDIUM TO LARGE ROOMS TURN ANY SPACE INTO AN APPLIANCE- BASED VIDEO CONFERENCING ROOM WIT ROOMMATE, AN APPLIANCE PURPOSE- BUILT FOR VIDEO COLLABORATION					
TV MOUNT FOR VIDEDO BARS	952-000041	ABOVE OR BELOW A TV OR MONITOR FOR					
IV MOONT FOR VIDEDO BARS	952-000041	A SLEEK INSTALLATION AND MINIMAL					
		FOOTPRINT					
RALLY BAR	960-001308						
RALLY DAN	360-001308	LARGE ROOMS					
		TURN ANY SPACE INTO AN APPLIANCE-					
		BASED VIDEO CONFERENCING ROOM WITH					
LOGITECH ROOMMATE	950-000081	ROOMMATE. AN APPLIANCE PURPOSE-					
		-					
LOGITECH M510 WIRELESS MOUSE	910-001822	FULL-SIZE MOUSE WITH EXTRA CONTROLS					
LOGITECH SIGNATURE K650	020 010000	WIRELESS KEYBOARD, EQUIPPED FOR					
WIRELESS KEYBOARD	920-010908	EVERDAY WORK AND COMFORT					
NOTE:	ALL A/V EQ	UIPMENT, WIRING, AND TESTING BY E/C					

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ELECTRICAL

DRAWN BY: SDM/NJG APPROVED BY: DESIGN PROJ: 12493.700 CONST PROJ: AS NOTED OCTOBER 2024

E-601